



January 19, 2017

Via Federal Express

Honorable Robert Stein, Chairman
And Members of the Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

Re: Docket No. 467 – Homeland Towers LLC (HT) and Celco Partnership d/b/a Verizon Wireless (Verizon)
Development & Management Plan- Tower Facility at 100 Pocono Road, Brookfield CT (CT777).

Dear Chairman Stein and Members of the Siting Council,

Homeland Towers ("HT") respectfully requests that you please accept for review and Council approval this Development & Management Plan ("D&M Plan") filing for the Facility as approved in Docket No. 467.

Tower, Compound & Other Equipment

Enclosed are fifteen (15) sets of 11"x17" Development & Management Plans being filed in accordance with the Council's Decision and Order dated October 13, 2016 ("Decision and Order"). Two full-sized sets of the Development & Management Plans are also enclosed. The D&M Plan incorporates a 150' monopole as provided for in the Siting Council's Decision and Order in this Docket. Verizon will mount twelve (12) panel antennas and (9) RRUs at a centerline of 146'. The Town of Brookfield will mount their public safety equipment which consists of (3) whip antennas and (1) microwave dish at mounting height of 154', (1) whip antenna at 75' and (1) microwave dish at 65' as depicted on the drawings prepared by All Points Technology Corporation. Since the October 13, 2016 Decision and Order, The Town of Brookfield has requested to add a dedicated space for a generator within the approved compound. All of the above equipment is depicted on the drawings prepared by All Points Technology. Attached please also find a geotechnical study from NOBIS Engineering as well as a structural design report from Ambor Structures for the tower and foundation. Specifications for the antennas and generator are also provided. Please note that the Structural provided is for future build out and the site plan drawings show the initial proposed configuration.

The proposed D&M Plan also includes construction plans for the site clearing, drainage, and erosion and sedimentation control measures consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control as amended.

Required Notifications

In accordance with the provisions of RCSA Section 16-50j-77, Homeland Towers hereby notifies the Council of its intention to begin site work immediately after Council approval of the D&M Plan. Construction of the tower and other site improvements will commence upon issuance of a local building permit. The supervisor for all construction related matters on this project is Christian Carmody, located at InSite Towers, 1199 North Fairfax Street, Suite 700, Alexandria, VA 22314 and can be reached by telephone at 617-595-7254.



HOMELAND TOWERS

We respectfully request that this matter be included on the Council's next available agenda for review and approval.
Thank you for your consideration of the enclosed.

Sincerely,

Raymond Vergati (cv)

Raymond Vergati
rv@homelandtowers.us

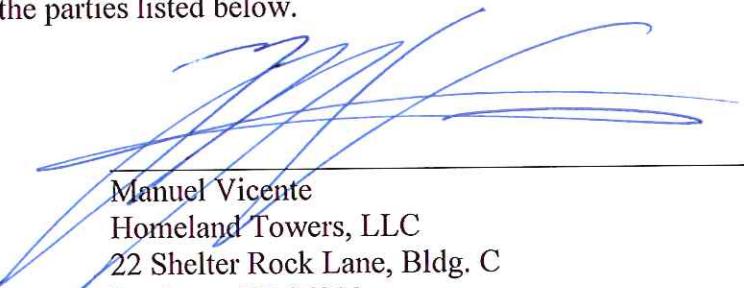
Enclosures

cc: Honorable Steve Dunn, First Selectman, Town of Brookfield
Manny Vicente, Homeland Towers LLC
Anthony Befera, Verizon
Scott Chasse, P.E., APT
Kenneth Baldwin, Esq., Cuddy & Feder LLP

CERTIFICATION OF SERVICE

I hereby certify that a copy of the foregoing D&M filing was sent by certified mail, return receipt requested, to each of the parties listed below.

Date: January 19, 2017


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Honorable Steve Dunn, First Selectman
Town of Brookfield
100 Pocono Road
Brookfield, CT 06804
sdunn@brookfieldct.gov

Geotechnical Engineering Report

**Proposed Homeland Towers: Brookfield CT-777
100 Pocono Road
Brookfield, Connecticut**

FOR

**All-Points Technology Corporation, P.C.
3 Saddlebrook Drive
Killingworth, CT 06419**

BY

**NOBIS ENGINEERING, INC.
122 CHURCH STREET
NAUGATUCK, CT 06770**

(203) 409-1292
www.nobiseng.com

Nobis Project No. 92230.00
NOVEMBER 5, 2016



November 5, 2016
File No. 92230.00

All-Points Technology Corporation, P.C.

Mr. Scott M. Chasse, P.E.
3 Saddlebrook Drive
Killingworth, CT 06419

Re: Transmittal of Geotechnical Engineering Report
Proposed Homeland Towers: Brookfield CT-777
100 Pocono Road, Brookfield, Connecticut

Dear Scott:

This report provides the results of Nobis Engineering Inc.'s (Nobis') geotechnical engineering review for the proposed telecommunications tower to be located at 100 Pocono Road in Brookfield, Connecticut. Our services were performed in general accordance with our October 12, 2016, *Geotechnical Engineering Services Proposal*. This report provides geotechnical recommendations for earthwork and foundation design for the proposed tower and associated equipment cabinets. The results of our field exploration program and geotechnical analyses are provided herein.

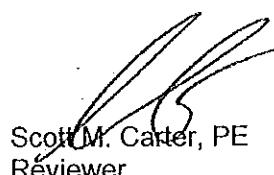
We appreciate the opportunity to work with you. Please call if you have any questions.

Sincerely,

NOBIS ENGINEERING, INC.



Raymond P. Janeiro, PE
Project Manager



Scott M. Carter, PE
Reviewer



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GEOTECHNICAL ENGINEERING REPORT
PROPOSED HOMELAND TOWERS: BROOKFIELD CT-777
100 POCONO ROAD
BROOKFIELD, CONNECTICUT

NOBIS FILE NO. 92230.00

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1.0 INTRODUCTION

Nobis Engineering, Inc. (Nobis) prepared this geotechnical engineering report for the proposed telecommunications tower and associated equipment foundations located at 100 Pocono Road in Brookfield, Connecticut ("Site") for All-Points Technology Corporation, P.C. ("Client"). A Site Locus Plan is included as **Figure 1**.

Existing conditions and proposed features relating to Site development are generally based on the Client's provided drawings (*Brookfield, 100 Pocono Road, sheets T-1, A-1, SP-1 through SP-3, prepared by All-Points Technology Corp., revision dated 05/26/2016*). Elevations referenced herein are based on Sheet SP-2 of the provided drawings, which states the Site elevation (EI) is near approximately EI 337 feet Above Mean Sea Level (AMSL). This report is subject to the limitations presented in **Appendix C**.

2.0 SITE AND PROJECT DESCRIPTIONS

2.1 Existing Site Conditions

The Site is located at the Town of Brookfield Department of Public Works materials yard and is generally bordered by U.S. Route 7 to the west, town-owned athletic fields to the north, a volunteer fire station to the east, and industrial/commercial parcels to the south. Existing site grades are relatively level at about EI 336± feet AMSL. Existing site conditions consist of an unpaved yard and material stockpiles. An apparent asphalt millings stockpile was recently relocated from the proposed compound area prior to completing our subsurface exploration program.

2.2 Proposed Construction

The project consists of constructing 150-foot monopole telecommunications tower and associated equipment cabinets within a 75-foot by 75-foot fenced compound with a gravel wearing surface. The estimated vertical load of the tower is approximately 60 kips, and bearing pressure for the equipment pads approximately 150 pounds per square foot (psf). It's anticipated that nominal cuts and fills on the order of 1-foot or less are anticipated to achieve design grade and that no significant slopes will be required. Refer to the Exploration Location Plan (**Figure 2 – Appendix A**) for additional proposed development details.

3.0 SUBSURFACE CONDITIONS

3.1 Geologic Information

We reviewed available subsurface/geological information in the vicinity of the project area (1:125,000 scale, *Surficial Materials Map of Connecticut, Janet Radway Stone, 1992* and *Bedrock Geology of the Danbury Quadrangle, Connecticut, James W. Clarke, 1958*).

The surficial material within the area of the proposed telecommunications compound is mapped as glacial till consisting of a variable mixture of gravel, sand, silt, and clay that is intermixed with cobbles and boulders. The underlying bedrock is classified as white, medium-grained calcite marble interlayered with schist or phyllite (Inwood Marble Formation).

3.2 Subsurface Exploration Program

The subsurface exploration program coordinated and logged by Nobis included the observation of one test boring (NB-1) and three test probes (NP-1 through NP-3). The test boring and probes were completed on October 19, 2016, by New England Boring Contractors, Inc. of Glastonbury, Connecticut. Test borings were completed to assess in-situ soil conditions, and, if encountered, groundwater and bedrock conditions at the Site. The explorations were field located using taped measurements referencing existing site features. The approximate, as-drilled exploration locations are depicted on the attached **Figure 2, Exploration Location Plan**.

The explorations were completed using a CME-75 truck-mounted drill rig utilizing drive and wash drilling methods to advance the test boring to a depth of approximately 25 feet (El. 311 feet) below ground surface (bgs). Upon encountering refusal at a depth of approximately 20 feet bgs, a rock core sample was obtained to aid in bedrock confirmation and evaluating the type and quality of bedrock. The core samples were drilled using a five foot, double-barrel, NQ-size core barrel. The core times were recorded every foot of core length and rock quality was determined using visual classification.

The test probes were advanced using 4-inch outside diameter solid-stem augers to refusal at depths ranging from approximately 5.5 to 14.5 feet bgs. Refusal is defined as the depth at which the augers are unable to be advanced further despite increasing the downward pressure and torque applied by the drill rig. It should be noted that auger refusal can result under a variety of

circumstances, including but not limited to: very dense soils; nested cobbles along the auger flights; boulders, or other obstructions; and decomposed, weathered, or unweathered bedrock.

Soil samples were obtained in the boring by split-spoon sampling procedures in general accordance with ASTM D-1586. The split-spoon sampling procedure utilizes a standard 2-inch O.D. split-barrel sampler that is driven into the bottom of the boring with a 140-pound hammer falling a distance of 30 inches. The number of blows required to advance the sampler the middle 12-inches of a typical 24-inch penetration is recorded as the Standard Penetration Resistance Value (N). The blows are indicated on the boring log at their depth of occurrence and provide an indication of the relative consistency of the material.

3.3 Generalized Subsurface Profile

Excluding the presence of fill, the explorations were generally consistent with published geologic mapping. The generalized subsurface profile in the area of the proposed telecommunications compound, as inferred from the subsurface exploration data, is summarized as follows:

- Fill: Loose to medium dense, dark brown, silty SAND with gravel, containing wood debris (SM)
– 4 to 6 feet thick (to about Elev. 332 to 330); over
- Glacial Till: Medium dense to dense, brown, silty SAND with gravel (SM)
– 1 to 10.5 feet thick (to about Elev. 330.5 to 321.5); over
- Decomposed Rock: Very dense, brown-white, well-graded SAND with gravel (SW)
– 13 feet thick in NB-1 (to about Elev. 316); over
- Marble Bedrock – Bedrock was cored below the Decomposed Rock stratum in NB-1. Bedrock was classified as moderately hard, moderately weathered, white/gray Marble. The core recovery and rock quality designation (RQD), was 100% and 72%, respectively, indicating a fair rock mass quality. Additionally, the core sample reacted strongly with hydrochloric acid.

Visual classifications of soil and rock, and conditions encountered at each exploration location can be found in the provided boring and test probes logs, included as Appendix B.

3.4 Groundwater

Groundwater levels were measured in the boreholes at the times and under the conditions stated on the logs. Water was encountered at about 19 feet bgs during drilling at NB-1; however, drive and wash drilling and rock coring introduced water to the borehole.

Our measurements were taken over a relatively short period of time and may not be indicative of the fluctuations in yearly groundwater levels. Groundwater levels will vary due to seasonal factors, temperature, precipitation, construction activity and other conditions which may be different from the time of the exploration program.

3.5 Soil Resistivity Testing

On October 19, 2016, Nobis field personnel conducted in-situ soil resistivity testing in accordance with accepted engineering practices using the Wenner electrode configuration. Electrodes were spaced at 5, 10, 20, 30 and 40 feet. Two approximately perpendicular resistivity lines were completed in the general vicinity of the proposed tower location. The approximate locations and orientations of the resistivity lines are shown on the attached Figure 2. The results of the resistivity tests are as follows:

<u>Electrode Spacing (ft)</u>	<u>Resistivity (ohm-cm)</u>	
	<u>Line 1</u>	<u>Line 2</u>
5	3,734	7,708
10	5,745	5,745
20	11,490	19,533
30	17,235	17,235
40	19,150	22,980

Field resistivity results may be influenced by boulders, concrete, foundations, and underground utilities within the test area. Resistivity results will also fluctuate depending on the degree of compaction, moisture content, constituent solubility, and temperature. Field resistivity values may also vary depending upon season, precipitation, and other conditions that may differ from those at the time of testing.

3.6 Karst Geology

The site is located over carbonate marble bedrock and may be susceptible to karst topography (e.g., fissures, caverns, etc.). Sinkhole activity, though uncommon in the New England area, may result from the dissolution of the underlying bedrock stratum. Construction in karst topography is always accompanied by some degree of risk for future ground subsidence and instability. We are available to further discuss this matter with the Owner or Design Team if there are concerns about associated risks with developing this Site.

4.0 GEOTECHNICAL DESIGN AND CONSTRUCTION RECOMMENDATIONS

4.1 Geotechnical Evaluation

Based on the results of our subsurface investigation, it is our opinion the proposed steel monopole telecommunications tower may be supported on a monolithic mat or a pier-and-pad foundation bearing on the glacial till, or compacted Structural Fill or Crushed Stone (see Section 4.5 Materials and Compaction) placed over the glacial till. Crushed stone, if used, should be separated from soil subgrades, excavation sidewalls and backfill using a nonwoven geotextile, such as Mirafi 140N or equivalent.

Alternatively, the proposed telecommunications tower may be supported on a drilled shaft foundation extending through overburden soils and weathered rock into competent bedrock. The proposed equipment platform and other ancillary structures may derive support from the glacial till. Design recommendations and construction considerations for the recommended foundation systems are presented in the following sections.

4.2 Seismic Design Recommendations

Based on the density/consistency of the soils encountered in the explorations, it is our opinion that Site soils are not considered susceptible to liquefaction. Seismic forces on foundations should be designed in accordance with the Connecticut State Building Code. We recommend using the following seismic design values based on the 2016 Connecticut State Building Code:

- Site Class: C;
- MCE Spectral Response Accelerations: $S_s = 0.208g$ and $S_1 = 0.066g$;
- Site Coefficients: $F_a = 1.2$ and $F_v = 1.7$; and
- Seismic Design Parameters: $S_{MS} = 0.250$ and $S_{M1} = 0.113$; $S_{DS} = 0.167$ and $S_{D1} = 0.075$.

4.3 Tower Foundation Design Recommendations

4.3.1 Shallow Foundation (Mat/Pad) Alternative

Nobis recommends a maximum net allowable bearing pressure of 6 kips per square foot (ksf). Foundations should be embedded a minimum of 42 inches below final grades for frost protection. The total settlement is anticipated to be less than 1 inch and differential settlement to be less than 0.5 inches. Foundation settlement will depend on the variations within the subsurface soil profile, the structural loading conditions, the embedment depth of the foundation, the thickness of compacted fill, and the quality of earthwork operations.

We recommend an ultimate passive pressure coefficient (K_p) of 3.0. Calculated passive pressures should be reduced by a minimum factor of safety of 3, to reflect the amount of movement required to mobilize the passive resistance. We also recommend an ultimate coefficient of sliding friction of 0.5. A factor of safety of at least 1.5 should be applied to calculated sliding resistance.

To summarize, we recommend the following static design parameters:

Description	Value
Maximum Net Allowable Bearing Pressure ¹	.6 kips per square foot
Minimum Embedment Below Finished Grade	42 inches
Estimated Total Settlement ²	1 inch
Estimated Differential Settlement ²	½ inch
Total Soil Unit Weight	125 pounds per cubic foot
Ultimate Passive Pressure Coefficient, K_p ³	3.0
Ultimate Coefficient of Sliding Friction ⁴	0.5

Uplift resistance for the tower foundation may be computed as the sum of the weight of the foundation element and the weight of the soil overlying the foundation. For this computation, we recommend using a soil unit weight of 100 pounds per cubic foot (pcf) for engineered fill.

4.3.2 Shallow Foundation (Mat/Pad) Construction Recommendations

The proposed mat/pad foundation and associated equipment areas should be cleared of existing structures and vegetation and grubbed; and existing cobbles, boulders, and any identifiable deleterious materials should be removed. Existing fill (including re-worked parent materials), and

any other unsuitable materials, must be removed from beneath footing zones of influence to the top of firm, natural glacial till prior to construction. The zone of influence is defined as the area within a line projecting outward and downward from the outside edges of the foundation at a 1H:1V slope. Footings shall bear on a prepared subgrade of firm natural soil, or compacted Structural Fill or Crushed Stone (over firm natural soil). Refer to Section 4.5 Materials and Compaction for material placement recommendations.

Excavations for foundation subgrades are anticipated to consist of native glacial till containing varying amounts of silt that will be easily disturbed when wet. Earthwork should be performed in dry conditions so that disturbance to foundation subgrades is limited. During earthwork, the Contractor should be responsible for protecting subgrades from the elements and maintaining the soils in a suitable state until completion of the project. Backfill should not be placed over a subgrade with standing water or that is frozen. Standing water, if present, should be removed and any soft and yielding soil should be removed prior to backfill placement. Excavations to subgrade levels should be performed using a smooth-edged bucket in order to minimize possible disturbance to the in-place subgrade soils.

Soil subgrades should be proof-rolled under the observation of a qualified Geotechnical Engineer with at least four (4) passes of a smooth-drum vibratory roller (minimum 8,000 pounds, minimum centrifugal force of 12,500 pounds) or, where approved by the geotechnical engineer, a vibratory plate compactor with a minimum of 2,500 pounds of centrifugal force. Any soft or loose zones identified during proof-rolling should be excavated and replaced with compacted Structural Fill, as necessary, and as required by the Geotechnical Engineer.

4.3.3 Deep Foundation (Drilled Shaft) Alternative

We recommend the following static design parameters for a drilled shaft foundation alternative:

Description	Value
Maximum Net Allowable Bearing Capacity Competent Bedrock ¹	10 kips per square foot (ksf)
<u>Ultimate Bond Values²</u> Glacial Till Decomposed Rock Competent Bedrock	See Note 2 See Note 2 75 pounds per square inch

Coefficient of Lateral Subgrade Reaction³	
Glacial Till	50 (z/D) kips per cubic foot (kcf)
Decomposed Rock	70 (z/D) kcf
Competent Bedrock	100 (z/D) kcf
Angle of Internal Friction	
Glacial Till	34
Decomposed Rock	36
Competent Bedrock	40
Total Soil Unit Weight	
Glacial Till	125 pounds per cubic foot (pcf)
Decomposed Rock	130 (pcf)
Competent Bedrock	140 (pcf)
Minimum Drilled Shaft Diameter	Diameter of Monopole Base
Allowable Deflection at Top of Shaft	0.5 inch
1.	End-bearing in a rock socket should be neglected for design due to the movement required to mobilize side resistance in a drilled shaft is less than the movement required to mobilize end-bearing resistance. Therefore, ultimate bond values should be used to design the drilled shaft foundation. The allowable end bearing capacity also assumes that loose, disturbed material has been removed from the base of the shaft.
2.	Due to the relatively shallow bedrock, side resistance from overburden soils and decomposed rock should be ignored in design for strain compatibility reasons. The uplift capacity should be based on the dead weight of the shaft and side resistance provided by the competent rock. It's assumed that applied loading will not have a significant Poissons-effect on the shaft.
3.	<i>z</i> represents the depth below ground surface (feet) and D is the diameter of the foundation element (feet).

We anticipate that the design length of the shaft will be primarily dependent on the embedment/lateral capacity required to resist live loading. The drilled shaft will be subject to tension loads and therefore should have reinforcing steel that extend through the entire length of the shaft.

4.3.4 Deep Foundation (Drilled Shaft) Construction Recommendations

Technical specifications should be prepared by the specialty Contractor that require detailed material and construction submittals and proof of experience in drilled shaft installation. The drilling method or combination of methods selected by the contractor should be submitted for review by the geotechnical engineer, prior to mobilization of drilling equipment.

A section of temporary casing may be required to reduce the likelihood of caving of the side walls of the shaft hole. Concrete should be placed by directing the concrete down the center of the shaft in order to reduce the likelihood of hitting the reinforcing steel and segregating. Groundwater, if encountered in the shaft, should be removed prior to placing concrete; alternatively, concrete may be placed by tremie methods.

4.4 Equipment Platform Foundations

The proposed equipment cabinets and accessory structures may be designed as slabs-on-grade bearing on a base course of at least 12-inches of compacted Structural Fill or Crushed Stone overlying densified native soils as described in **Section 4.3.2 Shallow Foundation (Mat/Pad) Construction Recommendations** above. Alternatively, subsurface conditions are favorable for construction of concrete pier foundations placed below existing fill, on natural, undisturbed glacial till.

4.4.1 Equipment Platform Slab-on-grade Foundations

We recommend a maximum net allowable bearing pressure of 2 kips per square foot (ksf) for slab design. Frost walls should be embedded a minimum of 42 inches below final grades for frost protection. Alternatively, dense insulation boards could be used under lightly loaded slabs-on-grade to reduce frost penetration.

The total settlement is expected to be less than 1 inch and differential settlement to be less than 0.5 inches. We recommend an ultimate coefficient of sliding friction of 0.5 (except if insulation boards are used to minimize frost penetration). A factor of safety of at least 1.5 should be applied to calculated sliding resistance.

A modulus of subgrade reaction, k_s of no greater than 200 pounds per cubic inch (pci) should be used for design of the slab. Note, however, that the value of k_s is for a one (1) square foot area. The k_s value should be adjusted for larger areas using the following expression:

$$\text{Modulus of Subgrade Reaction } (k_s) = k_{sl} [(B+1)/(2B)]^2$$

Where: k_s = Coefficient of vertical subgrade reaction for loaded area,

k_{sl} = Coefficient of vertical subgrade reaction for 1 x 1 square foot area, and

B = Width of load area (footing) in feet.

We recommend adopting the same construction recommendations in Section 4.3.2 above for preparation of slab-on-grade subgrades.

4.4.2 Equipment Platform Pier Foundations

We recommend a maximum allowable soil bearing capacity of 4 kips per square foot (ksf) for piers end bearing on glacial till. Based on anticipated loads and the recommended soil bearing capacity, the anticipated total and differential settlement is less than one inch and one-half inch, respectively. Bottom of piers must be constructed at a minimum depth of 42-inches below final site grades. We recommend a minimum pier diameter of 12 inches. Construction operations should be planned to mitigate disturbance to the final subgrade. The base of pier excavations should be free of water and loose soils prior to placing concrete.

4.5 Materials and Compaction

Recommended earthwork materials are as follows:

Structural Fill is to be used beneath footings and slabs-on-grade, and other areas as appropriate, or as directed by the Geotechnical Engineer or his/her representative. The material shall consist of hard, inert, durable particles of stone and coarse sand. It shall be free from ice and snow, roots, surface coatings, sod, loam, clay, rubbish, and other deleterious or organic matter, and shall conform to the following gradation requirements:

Sieve Size	Percent Passing by Weight
3-inch	100
½-inch	50-85
No. 4	40-75
No. 50	8-28
No. 200	0-10*

Crushed Stone shall consist of either angular fragments of crushed rock or durable crushed gravel stone, be reasonably free of loam, clay, or other deleterious or organic matter, and shall conform to the following gradation requirements:

Sieve Size	Percent Passing by Weight
¾-inch	100
½-inch	10-50
⅜-inch	0-20
No. 4	0-5

Structural Fill and Crushed Stone should be placed in loose lifts not exceeding 8 inches in depth, and compacted to at least 95 percent of its maximum dry density, and within 2% of optimum moisture content, as determined by ASTM D1557, Method C (Modified Proctor).

4.6 Additional Construction Considerations

Based on information obtained from the subsurface exploration program, the proposed foundations and slabs-on-grade will be constructed above the groundwater table and construction dewatering is not anticipated. Stormwater runoff should not be permitted to accumulate on/within exposed subgrades and the runoff should be directed away from the exposed subgrade areas.

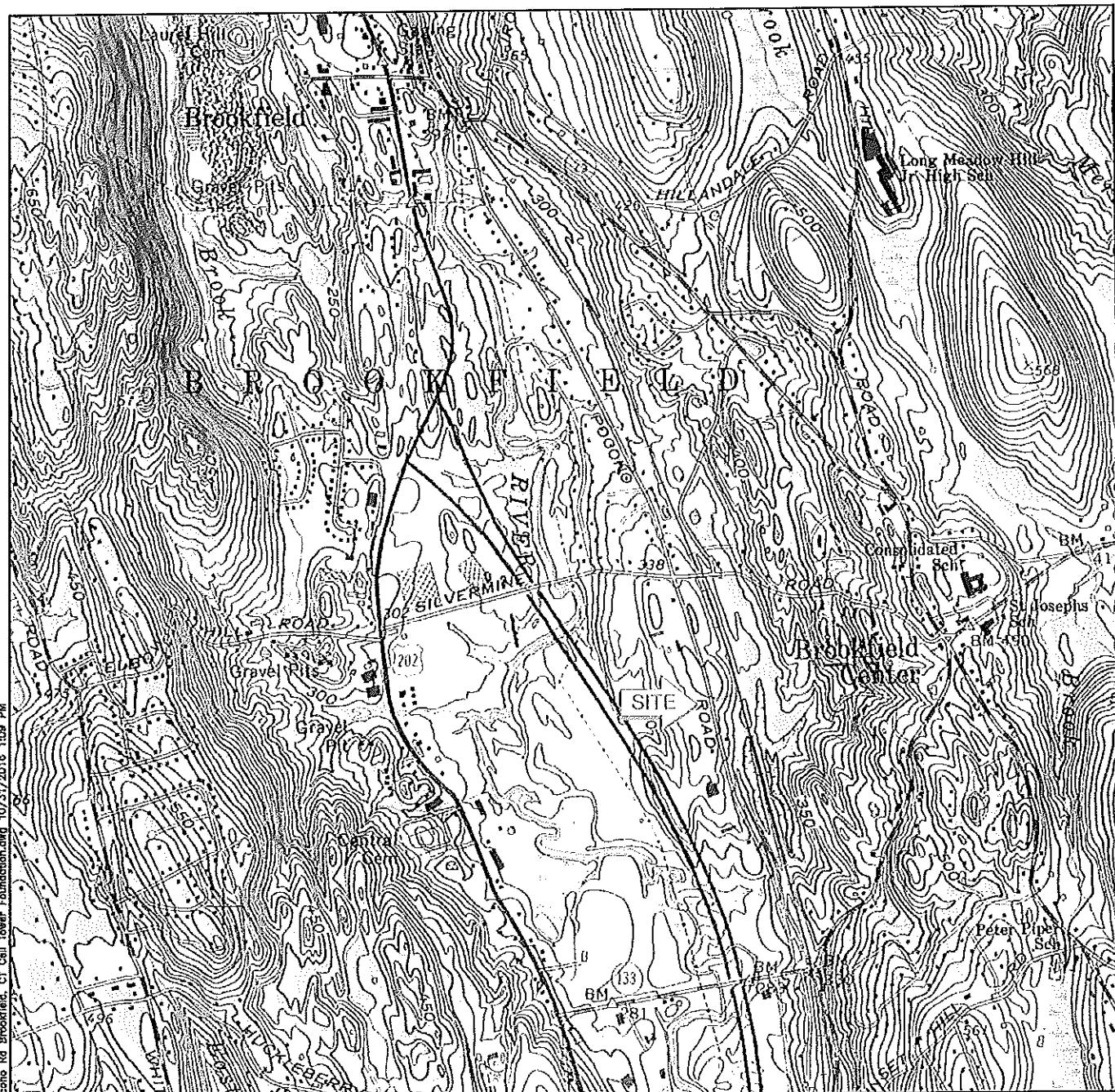
Where space permits and as needed, temporary slopes no steeper than 1.5H:1V appear to be appropriate. Excavation geometry should conform to OSHA excavation regulations contained in 29 CFR Part 1926. Temporary earth support will likely not be required for the excavations. If needed, temporary earth support systems should be designed by a Professional Engineer registered in the State of Connecticut.

We recommend that Nobis be provided the opportunity to review the final design plans and specifications to ensure that the recommendations of this report have been incorporated as intended. We further recommend that Nobis observes excavation to subgrade levels, subgrade preparation, and fill placement and compaction. This is recommended to evaluate and document the bearing material for the foundation subgrades. We also recommend that Nobis be retained to monitor the construction of the drilled shaft, if selected as the foundation alternative. The geotechnical engineer in the field should observe the work for compliance with the recommendations in this report, identify changes in subsurface conditions from those observed in the explorations should they become apparent, and assist in the development of design changes should subsurface conditions differ from those anticipated prior to the start of construction.

5.0 CLOSURE

This report is subject to the Limitations, included as Appendix C.

APPENDIX A – Figures



USGS TOPOGRAPHIC MAP

Brookfield, CT
October 2016

APPROX. SCALE IN FEET



NORTH



Engineering a Sustainable Future
Nobis Engineering, Inc.
122 Church Street
Naugatuck, CT 06770
T(203) 409-1292
www.nobisena.com

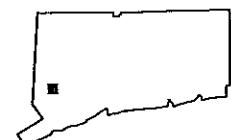
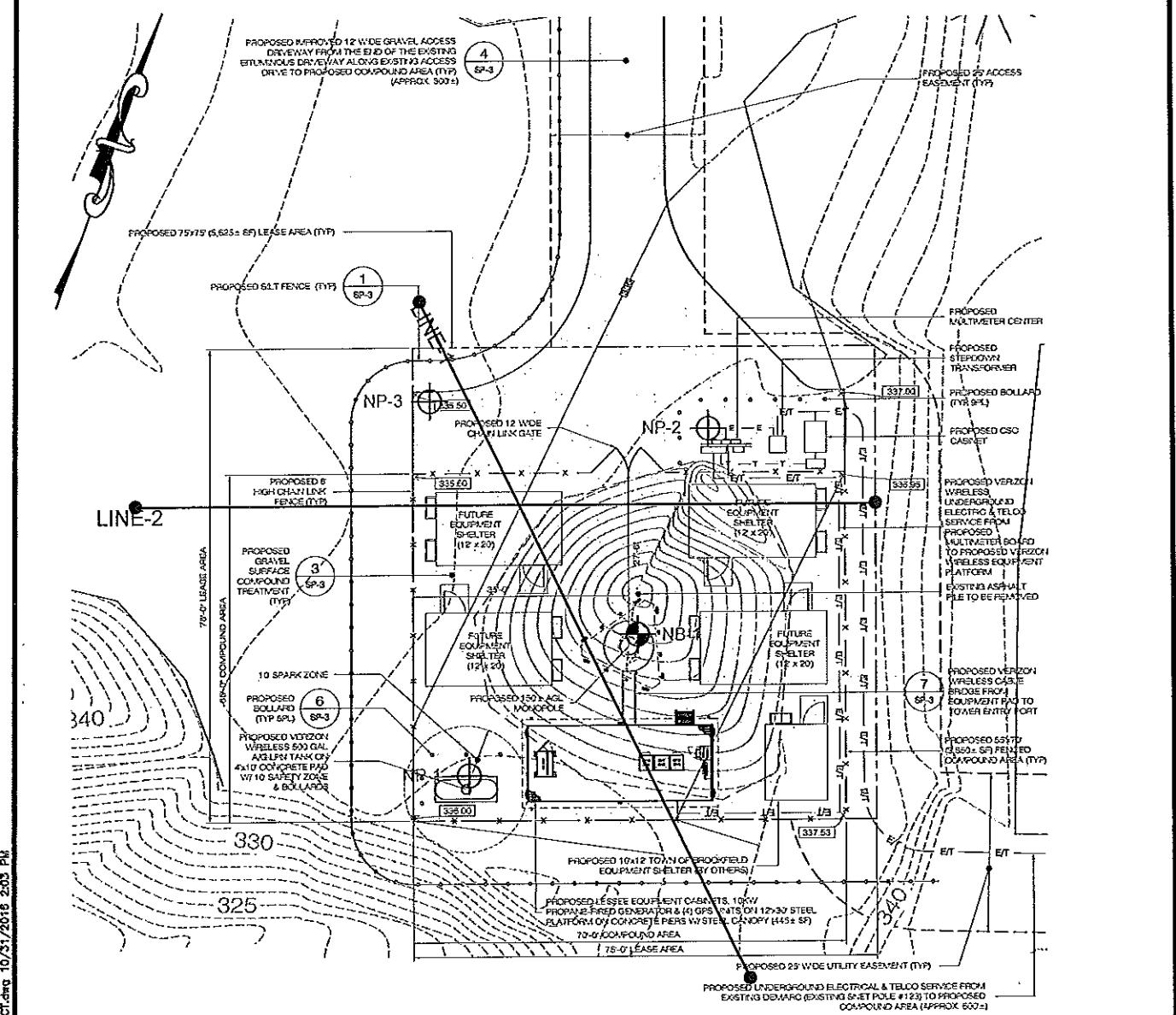


FIGURE 1

LOCUS PLAN
Proposed Homeland Towers, CT-777
100 Pocono Road
Brookfield, CT 06804



NOTES:

1. THE BASE PLAN WAS DEVELOPED FROM FIGURE ENTITLED, "COMPOUND PLAN AND TOWER ELEVATION," PREPARED BY ALL-POINTS TECHNOLOGY CORPORATION, P.C., REVISION DATED MAY 26, 2016.
2. TEST BORINGS AND PROBES WERE DRILLED BY NEW ENGLAND BORING CONTRACTORS, INC. OF GLASTONBURY, CONNECTICUT, AND OBSERVED BY NOBIS ON OCTOBER 19, 2016.
3. RESISTIVITY TESTING WAS PERFORMED ON OCTOBER 19, 2016 BY NOBIS FIELD STAFF.
4. THE APPROXIMATE LOCATIONS OF THE TEST BORING, TEST PROBES, AND RESISTIVITY TESTS WERE TAPED FROM EXISTING SITE FEATURES. THE LOCATIONS SHOULD BE CONSIDERED ACCURATE ONLY TO THE DEGREE IMPLIED BY THE METHOD USED.

LEGEND:

- NP-1 TEST PROBE LOCATION
- NB-1 TEST BORING LOCATION
- LINE-1 RESISTIVITY TEST LOCATION (TYP.)



Engineering a Sustainable Future
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APPROX. SCALE IN FEET
0 25' 50'

FIGURE 2

EXPLORATION LOCATION PLAN
Proposed Homeland Towers, CT-777
100 Pocono Road
Brookfield, CT 06804

DRAWN BY: JDV

APPENDIX B – Exploration Logs

 <p>BORING LOG</p>				<p>Boring No.: NB-1</p> <p>Boring Location: See Exploration Location Plan</p>																					
Project: <u>Proposed Homeland Towers Brookfield CT-777</u> Location: <u>100 Pocono Road Brookfield, CT</u> Nobis Project No.: <u>92230.00</u>				Checked by: <u>R. Janeiro</u> Date Start: <u>October 19, 2016</u> Date Finish: <u>October 19, 2016</u>																					
Contractor: <u>New England Boring Contractors</u> Driller: <u>Mike St. John</u> Nobis Rep.: <u>A. Epstein</u>				Rig Type / Model: <u>Truck / B-53 Mobile</u> Hammer Type: <u>Safety Hammer</u> Hammer Hoist: <u>Wire Winch</u>																					
				Ground Surface Elev.: <u>(+/-) 336</u> Datum: <u>AMSL</u>																					
		Drilling Method		Sampler		Groundwater Observations																			
Type	Casing		Split-Spoon		Date	Time	Depth Below Ground (ft.)	Depth of Casing (ft.)	Depth to Bottom of Hole (ft.)	Stabilization Time															
Size ID (in.)	4		1-3/8		10/19/16	12:30	19.0	N/A	24.65	2 hr															
Advancement	Drive and Wash		140-lb Hammer																						
Depth (ft.)	SAMPLE INFORMATION				LITHOLOGY				SAMPLE DESCRIPTION AND REMARKS (Classification System: Modified Buhrmaster)			NOTES													
	Type & No.	Rec (in.)	Depth (ft.)	Blows/6 in.	REC %	RQD %	Drilling Rate (min/ft.)	Ground Water					Stratum Elev. / Depth (ft.)												
1	S-1	14	0-2	8																					
2	S-2	8	2-4	5																					
3	S-3	12	5-7	10																					
4	S-4	8	7-9	24																					
5	S-5	14	10-12	9																					
6	S-6	7	15-17	12																					
7	RC-1	60	20-25	100/0"	50/0"	0.1																			
8																									
9																									
10																									
11																									
12																									
13																									
14																									
15																									
16																									
17																									
18																									
19																									
20																									
21																									
22																									
Soil Percentage Non-Soil NOTES: <tr> <td>trace</td> <td>5 - 10</td> <td>very few</td> <td colspan="10">1) Boring was backfilled with cuttings.</td> </tr>													trace	5 - 10	very few	1) Boring was backfilled with cuttings.									
trace	5 - 10	very few	1) Boring was backfilled with cuttings.																						

 <p>Nobis Engineering a Sustainable Future</p>				<h3>BORING LOG</h3> <p>Boring No.: NB-1 Boring Location: See Exploration Location Plan</p>						
Project: Proposed Homeland Towers Brookfield CT-777 Location: 100 Pocono Road Brookfield, CT Nobis Project No.: 92230.00				Checked by: R. Janeiro Date Start: October 19, 2016 Date Finish: October 19, 2016						
Contractor: New England Boring Contractors Driller: Mike St. John Nobis Rep.: A. Epstein		Rig Type / Model: Truck / B-53 Mobile Hammer Type: Safety Hammer Hammer Hoist: Wire Winch		Ground Surface Elev.: (+/-) 336 Datum: AMSL						
				Groundwater Observations						
Type	Casing	Sampler		Date	Time	Depth Below Ground (ft.)	Depth of Casing (ft.)	Depth to Bottom of Hole (ft.)	Stabilization Time	
		Split-Spoon		▼ 10/19/16	12:30	19.0	N/A	24.65	2 hr	
Size ID (in.)	4	1-3/8								
Advancement	Drive and Wash	140-lb Hammer								
Depth (ft.)	SAMPLE INFORMATION			REC %	Drilling Rate (min/ft)	Ground Water	LITHOLOGY	SAMPLE DESCRIPTION AND REMARKS (Classification System: Modified Burmister)		NOTES
23	Type & No.	Rec (in.)	Depth (ft.)	Blows/6 in.	RQD %	Graphic	Stratum Elev. / Depth (ft.)			
24										
25										
26										
27										
28										
29										
30										
31										
32										
33										
34										
35										
36										
37										
38										
39										
40										
41										
42										
43										
44										
45										
Soil	Percentage	Non-Soil	NOTES:							
trace	5 - 10	very few	1) Boring was backfilled with cuttings.							

 <p>BORING LOG</p> <p>Project: <u>Proposed Homeland Towers Brookfield CT-777</u></p> <p>Location: <u>100 Pocono Road Brookfield, CT</u></p> <p>Nobis Project No.: <u>92230.00</u></p>				Boring No.: <u>NP-1</u> Boring Location: See Exploration Location Plan							
<p>Contractor: <u>New England Boring Contractors</u></p> <p>Driller: <u>Mike St. John</u></p> <p>Nobis Rep.: <u>A. Epstein</u></p>				Checked by: <u>R. Janeiro</u> Date Start: <u>October 19, 2016</u> Date Finish: <u>October 19, 2016</u>							
<p>Rig Type / Model: <u>Truck / B-53 Mobile</u></p> <p>Hammer Type: <u>N/A</u></p> <p>Hammer Hoist: <u>N/A</u></p>				Ground Surface Elev.: <u>(+/-) 336</u> Datum: <u>AMSL</u>							
		Drilling Method	Sampler		Groundwater Observations						
Type		Solid Stem Auger	N/A		Date	Time	Depth Below Ground (ft.)	Depth of Casing (ft.)	Depth to Bottom of Hole (ft.)	Stabilization Time	
					10/19/16	12:30	Not Encountered	N/A	9.5	10 min	
Size ID (in.)		4 (O.D.)	N/A								
Advancement		Augered	N/A								
Depth (ft.)	SAMPLE INFORMATION				SAMPLE DESCRIPTION AND REMARKS (Classification System: Modified Buhrmaster)						NOTES
	Type & No.	Rec (in.)	Depth (ft.)	Blows/6 in.	Ground Water	Graphic	Stratum Elev./Depth (ft.)				
1											
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
Soil	Percentage	Non-Soil	NOTES:								
trace	5 - 10	very few	1) End of exploration at approximately 9.5 feet below ground surface after SSA refusal on inferred weathered rock. 2) Borehole has filled with cuttings.								

Nobis				BORING LOG			Boring No.: NP-2				
Engineering a Sustainable Future				Project: Proposed Homeland Towers Brookfield CT-777			Boring Location: See Exploration Location Plan				
				Location: 100 Pocono Road Brookfield, CT			Checked by: R. Janeiro				
				Nobis Project No.: 92230.00			Date Start: October 19, 2016				
							Date Finish: October 19, 2016				
Contractor: New England Boring Contractors		Rig Type / Model: Truck / B-53 Mobile		Ground Surface Elev.: (+/-) 336							
Driller: Mike St. John		Hammer Type: N/A									
Nobis Rep.: A. Epstein		Hammer Hoist: N/A		Datum: AMSL							
		Drilling Method		Sampler		Groundwater Observations					
Type		Solid Stem Auger		N/A		Date	Time	Depth Below Ground (ft.)	Depth of Casing (ft.)	Depth to Bottom of Hole (ft.)	Stabilization Time
10/19/16		12:30		Not Encountered		N/A		5.5		10 min	
Size ID (in.)		4 (O.D.)		N/A							
Advancement		Augered		N/A							
Depth (ft.)	SAMPLE INFORMATION				LITHOLOGY		SAMPLE DESCRIPTION AND REMARKS (Classification System: Modified Burmister)				NOTES
	Type & No.	Rec (in.)	Depth (ft.)	Blows/6 in.	Ground Water	Graphic	Stratum Elev. / Depth (ft.)				
1											
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											
17											
18											
19											
20											
21											
22											
Soil	Percentage	Non-Soil	NOTES:								
trace	5 - 10	very few									
1) End of exploration at approximately 5.5 feet below ground surface after SSA refusal on inferred weathered rock.											

				BORING LOG				Boring No.: <u>NP-3</u> Boring Location: See Exploration Location Plan			
Project: <u>Proposed Homeland Towers Brookfield CT-777</u> Location: <u>100 Pocono Road Brookfield, CT</u> Nobis Project No.: <u>92230.00</u>											
Contractor: <u>New England Boring Contractors</u> Driller: <u>Mike St. John</u> Nobis Rep.: <u>A. Epstein</u>				Rig Type / Model: <u>Truck / B-53 Mobile</u> Hammer Type: <u>N/A</u> Hammer Hoist: <u>N/A</u>				Ground Surface Elev.: <u>(+/-) 336</u> Datum: <u>AMSL</u>			
				Groundwater Observations							
Type		Solid Stem Auger		<u>N/A</u>		Date	Time	Depth Below Ground (ft.)	Depth of Casing (ft.)	Depth to Bottom of Hole (ft.)	Stabilization Time
						<u>10/19/16</u>	<u>12:30</u>	<u>Not Encountered</u>	<u>N/A</u>	<u>14.5</u>	<u>10 min</u>
Size ID (in.)		4 (O.D.)		<u>N/A</u>							
Advancement		Augered		<u>N/A</u>							
Depth (ft.)	SAMPLE INFORMATION			LITHOLOGY			SAMPLE DESCRIPTION AND REMARKS (Classification System: Modified Burmister)				NOTES
	Type & No.	Rec (in.)	Depth (ft.)	Blows/6 in.	Ground Water	Graphics					
1							Dark brown, fine to medium SAND, little Silt, trace fine Gravel, very few wood debris. dry. (FILL).				
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15							Boring terminated at 14.5 feet.				
16											
17											
18											
19											
20											
21											
22											
Soil	Percentage	Non-Soil	NOTES:								
trace	5 - 10	very few	1) End of exploration at approximately 14.5 feet below ground surface after SSA refusal on inferred weathered rock. 2) Depth to bottom of hole = 14.5 ft.								

APPENDIX C – Limitations

GEOTECHNICAL LIMITATIONS

Explorations and Subsurface Conditions

1. The analyses and design recommendations submitted in this report are based in part upon the data obtained from subsurface explorations. The nature and extent of variations between these explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.

In preparing this report, Nobis relied on certain information provided by the Client and other parties referenced therein which were made available to Nobis at the time of our evaluation. Nobis did not attempt to independently verify the accuracy or completeness of all information reviewed or received during the course of this evaluation.

2. The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretations of widely spaced explorations and samples; actual soil transitions are probably more erratic. For specific information, refer to the exploration logs.

3. Water level readings have been made in the explorations at times and under conditions stated on the logs. These data have been reviewed and interpretations have been made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, and other factors occurring since the time measurements were made. The water table encountered in the course of the work may differ from that indicated in the Report.

Recommendations for foundation drainage, waterproofing, and moisture control address the conventional geotechnical engineering aspects of seepage control. These recommendations may not preclude an environment that allows the infestation of mold or other biological pollutants.

4. Nobis' geotechnical services did not include an assessment of the presence of oil or hazardous materials at the property. Consequently, we did not consider the potential impacts (if any) that contaminants in soil or groundwater may have on construction activities, or the use of structures on the property.

Additional Services

5. Nobis recommends that we be retained to provide services during future site observations, design, implementation activities, construction and/or property development/redevelopment. This will allow us the opportunity to: i) observe conditions and compliance with our recommendations, design concepts and/or opinions; ii) allow for changes in the event that conditions are other than anticipated; iii) provide modifications to our design recommendations; and iv) assess the consequences of changes in technologies and/or regulations.

Use of Report

6. Nobis prepared this report on behalf of, and for the exclusive use of our Client for the stated purpose(s) and location(s) identified in our proposal and/or report. Use of this report, in whole or in part, at other locations, or for other purposes, may lead to inappropriate conclusions; and we do not accept any responsibility for the consequences of such use(s). Reliance by any party not expressly identified in the agreement, for any use, without our prior written permission, shall be at that party's sole risk, and without any liability to Nobis.

This report is for design purposes only and is not sufficient to prepare an accurate construction bid. Contractors wishing a copy of the report may secure it with the understanding that its scope is limited to design considerations only.

7. Nobis' findings and conclusions are based on the work conducted as part of the scope of work set forth in our proposal and/or report, and reflect our professional judgment. These findings and conclusions must be considered not as scientific or engineering certainties, but rather as our professional opinions considering the limited data gathered during the course of our work. If conditions other than those described in this report are found at the subject location(s), or the project design has been altered in any way, Nobis shall be so notified and afforded the opportunity to revise the report, as appropriate, to reflect the unanticipated changed conditions.

8. Nobis' services were performed using the degree of skill and care ordinarily exercised by qualified professionals performing the same type of services, at the same time, under similar conditions, at the same or a similar property. No warranty, expressed or implied, is made.

Compliance with Codes and Regulations

9. Nobis used reasonable care in identifying and interpreting applicable codes and regulations. These codes and regulations are subject to various, and possibly contradictory, interpretations. Compliance with codes and regulations by other parties is beyond our control.

Opinion of Cost

10. This report may contain or be based on comparative cost opinions for the purpose of evaluating alternative foundation schemes. These opinions may also involve approximate quantity evaluations. It should be noted that quantity estimates may not be accurate enough for construction bids. In addition, since we are not professional estimators of labor and materials cost, the evaluation of construction costs should be considered as approximate guidelines and could vary significantly from actual costs. Nobis does not guarantee the accuracy of our cost opinions as compared to contractor's bids for construction costs.



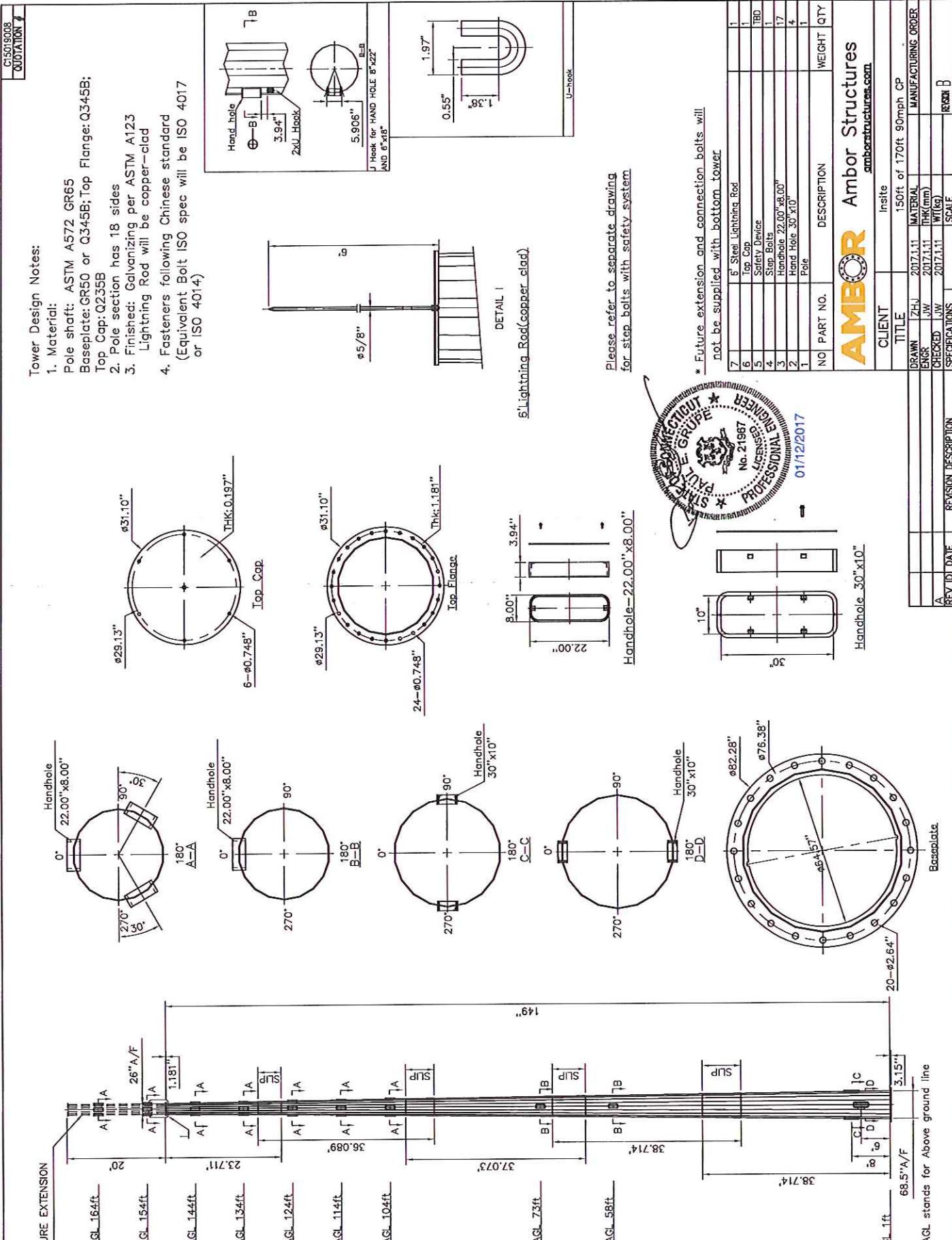
Structural Design Report

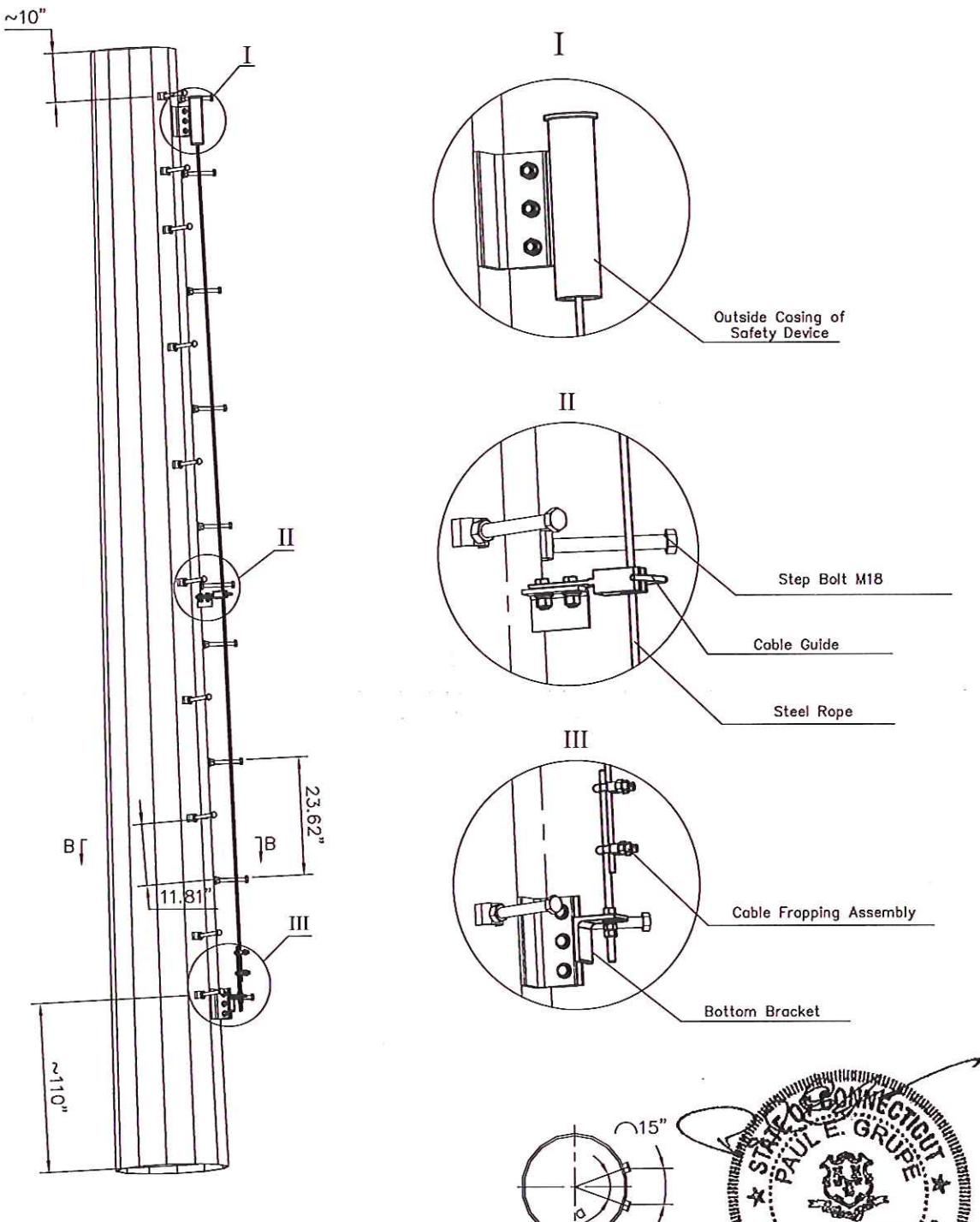
Date: January 12, 2016
Customer: Insite Wireless Group, LLC
Project: 150' of 170' Monopole
Site: Brookfield, CT
Site Number: CT777
Ambor Job Number: C15019008

Monopole Profile Rev G.....	3~4
Foundation Drawing.....	5
Pole Calculation Rev G.....	6~40
Foundation Calculation Rev G.....	41

Prepared by Vince Jiang

Ambor Structures, Inc.





Note:

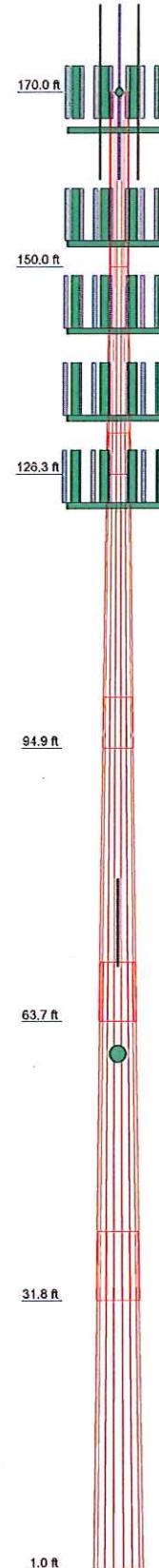
1. Arc length between two sides step bolt brackets is around 15". The angle "a" will be no bigger than 120°
2. Vertical distance between two step bolts on same side is around 23.62"
3. Vertical distance between two step bolts on different sides is around 11.81"

AMBOR

Ambor Structures
amborstructures.com

CLIENT

Section	Length (ft)	5	4	3	2	1
	388-5/8"	389-17/32"	3727/32"	361-3/32"	239-5/8"	20'
Number of Sides	18	18	18	18	18	18
Thickness (in)	0.43	0.39	0.39	0.31	0.24	0.20
Socket Length (ft)	7'11-1/32"	6'9-15/32"	5'10-3/32"	4'9-1/8"		
Top Dia (in)	56.76	48.21	39.81	31.26	25.98	
Bot Dia (in)	68.50	59.95	51.05	42.21	33.18	25.98
Grade		A572-65				
Weight (K)	34.5	11.3	8.8	7.1	4.5	1.8
						1.1



MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 90 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0'.
8. Installation per TIA/EIA-222 and AISC Specifications.
9. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards. Assume that there will be a one (1) foot caisson reveal
10. TOWER RATING: 96.7%

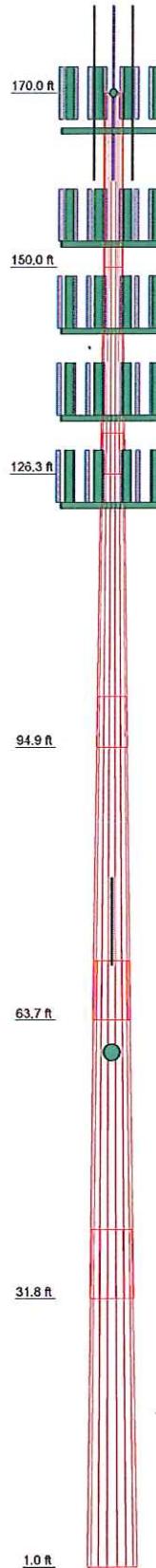
ALL REACTIONS
ARE FACORED

AXIAL
206 K
SHEAR
15 K
MOMENT
1950 kip-ft
TORQUE 0 kip-ft
50 mph WIND - 0.75 in ICE

AXIAL
73 K
SHEAR
55 K
MOMENT
7148 kip-ft
TORQUE 0 kip-ft
REACTIONS - 90 mph WIND



Section	Length (ft)	Number of Sides	Thickness (in)	Socket Length (ft)	Top Dia (in)	Bot Dia (in)	Grade	Weight (k)
6	388-5/16"	18	0.39	711-1/32"	48.21	59.95	A572-65	8.8
5	388-17/32"	18	0.39	69-15/32"	39.81	51.06		7.1
4	3727/32"				31.26	42.21		4.5
3	361-3/32"				33.18			1.8
2	238-5/8"				25.98			1.1
1	20'							



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
21.9' Omni Antenna	170	(8) Ericsson RRUS 11	146
21.9' Omni Antenna	170	Raycap DC6-48-60-18-8F	146
21.9' Omni Antenna	170	(2) Ericsson RRUS A2 Module	146
1' HP	170	12.6' Low Profile Platform w/Rails Sector	146
(4) Generic 96"x12"x7" Panel	166	(4) Generic 96"x12"x7" Panel	136
(4) Ericsson RRUS 11	166	(4) Ericsson RRUS 11	136
(2) Raycap DC6-48-60-18-8F	166	Raycap DC6-48-60-18-8F	136
(2) Ericsson RRUS A2 Module	166	(2) PCTEL GPS-TMG-HR-26N 5"x3.2"	136
12.6' Low Profile Platform w/Rails Sector	166	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	136
(4) Generic 96"x12"x7" Panel	166	12.6' Low Profile Platform w/Rails Sector	136
(4) Ericsson RRUS 11	166	(4) Generic 96"x12"x7" Panel	136
Raycap DC6-48-60-18-8F	166	(4) Ericsson RRUS 11	136
(2) Ericsson RRUS A2 Module	166	Raycap DC6-48-60-18-8F	136
12.6' Low Profile Platform w/Rails Sector	166	PCTEL GPS-TMG-HR-26N 5"x3.2"	136
(4) Generic 96"x12"x7" Panel	166	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	136
(4) Ericsson RRUS 11	166	12.6' Low Profile Platform w/Rails Sector	136
Raycap DC6-48-60-18-8F	166	(4) Generic 96"x12"x7" Panel	136
(2) Ericsson RRUS A2 Module	166	(4) Ericsson RRUS 11	136
12.6' Low Profile Platform w/Rails Sector	166	Raycap DC6-48-60-18-8F	136
(4) Generic 96"x12"x7" Panel	156	PCTEL GPS-TMG-HR-26N 5"x3.2"	136
(4) Ericsson RRUS 11	156	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	136
Raycap DC6-48-60-18-8F	156	12.6' Low Profile Platform w/Rails Sector	136
(2) Ericsson RRUS A2 Module	156	(4) Generic 96"x12"x7" Panel	136
12.6' Low Profile Platform w/Rails Sector	156	(4) Ericsson RRUS 11	136
(4) Generic 96"x12"x7" Panel	156	Raycap DC6-48-60-18-8F	136
(4) Ericsson RRUS 11	156	PCTEL GPS-TMG-HR-26N 5"x3.2"	136
Raycap DC6-48-60-18-8F	156	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	136
(2) Ericsson RRUS A2 Module	156	12.6' Low Profile Platform w/Rails Sector	136
12.6' Low Profile Platform w/Rails Sector	156	(4) Generic 96"x12"x7" Panel	136
(4) Generic 96"x12"x7" Panel	156	(4) Ericsson RRUS 11	136
(4) Ericsson RRUS 11	156	Raycap DC6-48-60-18-8F	136
Raycap DC6-48-60-18-8F	156	PCTEL GPS-TMG-HR-26N 5"x3.2"	136
(2) Ericsson RRUS A2 Module	156	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	136
12.6' Low Profile Platform w/Rails Sector	156	12.6' Low Profile Platform w/Rails Sector	136
(4) Generic 96"x12"x7" Panel	146	(4) Generic 96"x12"x7" Panel	126
(8) Ericsson RRUS 11	146	(4) Ericsson RRUS 11	126
(2) Raycap DC6-48-60-18-8F	146	(4) E15S09P78 (TMA)	126
(2) Ericsson RRUS A2 Module	146	12.6' Low Profile Platform w/Rails Sector	126
12.6' Low Profile Platform w/Rails Sector	146	(4) Generic 96"x12"x7" Panel	126
(4) Generic 96"x12"x7" Panel	146	(4) Ericsson RRUS 11	126
(8) Ericsson RRUS 11	146	Raycap DC6-48-60-18-8F	126
(2) Raycap DC6-48-60-18-8F	146	(4) E15S09P78 (TMA)	126
(2) Ericsson RRUS A2 Module	146	12.6' Low Profile Platform w/Rails Sector	126
12.6' Low Profile Platform w/Rails Sector	146	(4) Generic 96"x12"x7" Panel	126
(4) Generic 96"x12"x7" Panel	146	(4) E15S09P78 (TMA)	126
(4) Ericsson RRUS 11	146	12.6' Low Profile Platform w/Rails Sector	126
Raycap DC6-48-60-18-8F	146	(2) Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	126
(2) Ericsson RRUS A2 Module	146	12.6' Low Profile Platform w/Rails Sector	126
12.6' Low Profile Platform w/Rails Sector	146	(4) Generic 96"x12"x7" Panel	126
(4) Generic 96"x12"x7" Panel	146	(4) E15S09P78 (TMA)	126
(4) Ericsson RRUS 11	146	21.9' Omni	75
Raycap DC6-48-60-18-8F	146	2ft Dish	60
(2) Ericsson RRUS A2 Module	146	(4) Generic 96"x12"x7" Panel	146

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

ALL REACTIONS ARE FACORED

AXIAL
206 K

SHEAR
15 K

MOMENT
1950 k/⁷

TORQUE 0 kip-ft

50 mph WIND - 0.75 in ICE

AXIAL
73 K

SHEAR
55 K

MOMENT
7148 kip-ft

TORQUE 0 kip-ft

REACTIONS - 90 mph WIND

1. Tower is located in Fairfield County, Connecticut.

2. Tower designed for Exposure C to the TIA-222-G Standard.

3. Tower designed for a 90 mph basic wind in accordance with the TIA-222-G Standard.

4. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.

5. Deflections are based upon a 60 mph wind.

6. Tower Structure Class II.

7. Topographic Category 1 with Crest Height of 0'

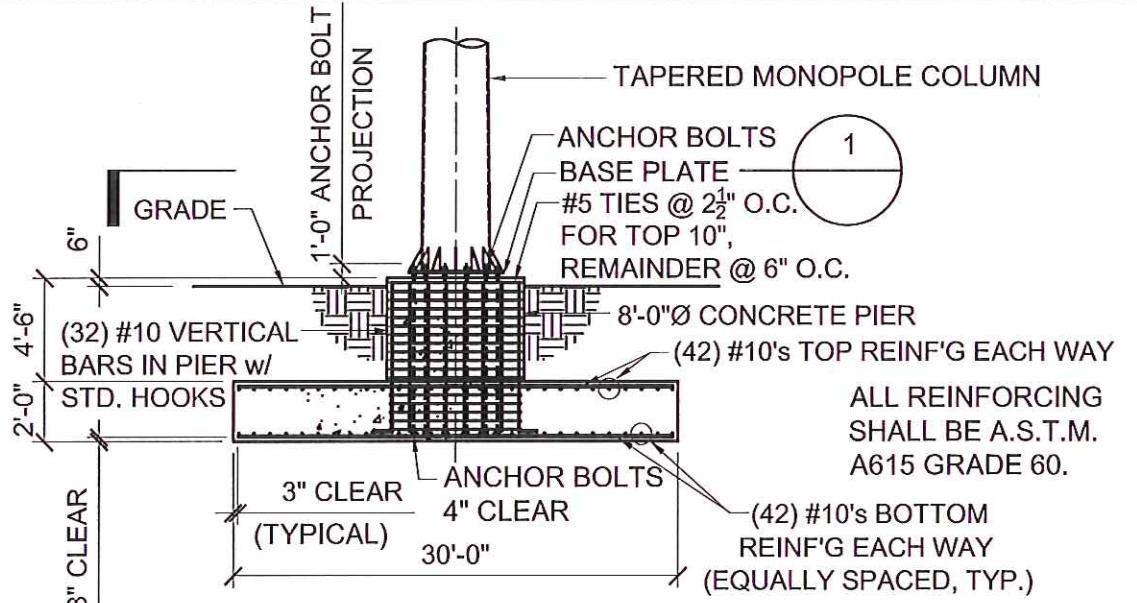
8. Installation per TIA/EIA-222 and AISC Specifications.

9. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards. Assume that there will be a one (1) foot caisson reveal

10. TOWER RATING: 96.7%



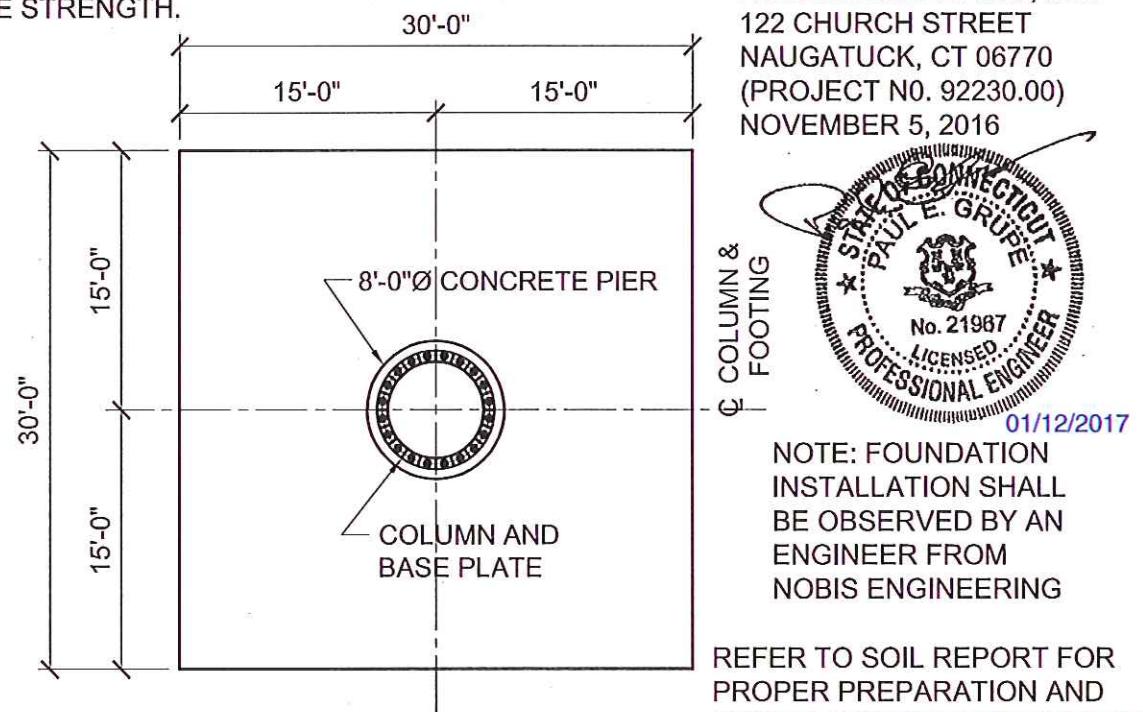
01/12/2017



SPREAD FOUNDATION

NOT TO SCALE

NOTE: CONCRETE SHALL BE
3000 P.S.I. (MINIMUM) @ 28 DAYS
COMPRESSIVE STRENGTH.



B&P JOB NO. 16700.021

SECTION
1
SCALE: $\frac{3}{32}$ " = 1'-0"

AMBOR
STRUCTURES

bennett & pless

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1300 Riverfront Parkway Suite 105

job name	MONPOLE CELL TOWER	date	1/12/17	sheet #
SPREAD FOUNDATION w / PIER		TED	1	

tnxTower	Job 150ft of 170ft.90mph	Page 1 of 33
Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Project Brookfield CT	Date 17:04:39 01/11/17
	Client Ambor	Designed by Chunhui Song

Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 90 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0'.

Nominal ice thickness of 0.75 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards. Assume that there will be a one (1) foot caisson reveal.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	✓ Calculate Redundant Bracing Forces
Consider Moments - Diagonals	✓ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	✓ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
✓ Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
✓ Use Code Safety Factors - Guys	Retention Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	Bypass Mast Stability Checks	Consider Feed Line Torque
Always Use Max Kz	✓ Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	✓ Project Wind Area of Appurt.	Use TIA-222-G Bracing Resist. Exemption
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	✓ Use TIA-222-G Tension Splice Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Poles
Secondary Horizontal Braces Leg	Sort Capacity Reports By Component	✓ Include Shear-Torsion Interaction
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
SR Members Are Concentric		

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
		ft	ft	Sides	in	in	in	in	

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Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
L1	170'-150'	20'	0'	18	25.98	25.98	0.20	0.79	A572-65 (65 ksi)
L2	150'-126'3-3/8"	23'8-5/8"	4'9-1/8"	18	25.98	33.18	0.24	0.94	A572-65 (65 ksi)
L3	126'3-3/8"-94'11 -13/32"	36'1-3/32"	5'10-3/32"	18	31.26	42.21	0.31	1.26	A572-65 (65 ksi)
L4	94'11-13/32"-63' 8-5/8"	37'27/32"	6'9-15/32"	18	39.81	51.06	0.39	1.57	A572-65 (65 ksi)
L5	63'8-5/8"-31'9-1 5/32"	38'8-17/32"	7'11-1/32"	18	48.21	59.95	0.39	1.57	A572-65 (65 ksi)
L6	31'9-15/32"-1"	38'8-5/8"		18	56.76	68.50	0.43	1.73	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ³	w in	w/t
L1	26.39	16.11	1353.64	9.15	13.20	102.55	2709.06	8.06	4.23	21.472
	26.39	16.11	1353.64	9.15	13.20	102.55	2709.06	8.06	4.23	21.472
L2	26.39	19.30	1616.94	9.14	13.20	122.50	3236.00	9.65	4.16	17.6
	33.69	24.70	3385.86	11.69	16.85	200.89	6776.18	12.35	5.42	22.959
L3	33.21	30.94	3743.35	10.99	15.88	235.71	7491.62	15.47	4.95	15.709
	42.86	41.88	9289.02	14.87	21.44	433.18	18590.26	20.95	6.88	21.828
L4	42.22	49.26	9668.23	13.99	20.22	478.07	19349.19	24.63	6.31	16.037
	51.84	63.31	20527.82	17.98	25.94	791.48	41082.66	31.66	8.29	21.064
L5	51.04	59.75	17257.41	16.97	24.49	704.69	34537.54	29.88	7.79	19.791
	60.87	74.42	33347.87	21.14	30.45	1095.02	66739.63	37.22	9.86	25.04
L6	60.07	77.42	31031.79	20.00	28.83	1076.26	62104.42	38.72	9.23	21.306
	69.56	93.57	54775.62	24.17	34.80	1574.01	109623.32	46.79	11.29	26.08

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
L1 170'-150'				1	1	1			
L2				1	1	1			
150'-126'3-3/8"									
L3				1	1	1			
126'3-3/8"-94'1 1-13/32"									
L4				1	1	1			
94'11-13/32"-6 3'8-5/8"									
L5				1	1	1			
63'8-5/8"-31'9- 15/32"									
L6				1	1	1			
31'9-15/32"-1"									

Monopole Base Plate Data

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Base Plate Data	
Base plate is square	
Base plate is grouted	✓
Anchor bolt grade	A615-75
Anchor bolt size	2.25 in
Number of bolts	20
Embedment length	98.43 in
f_c	6.00 ksi
Grout space	2.00 in
Base plate grade	A572-50
Base plate thickness	3.15 in
Bolt circle diameter	76.38 in
Outer diameter	82.28 in
Inner diameter	64.57 in
Base plate type	Plain Plate

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C _{A,A}	Weight
						ft	
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	170' - 5'	4	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	166' - 5'	12	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	156' - 5'	14	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	146' - 5'	12	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	136' - 5'	20	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	126' - 5'	14	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	75' - 5'	1	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00
LDF-50A (1 5/8 FOAM)	C	No	CaAa (Out Of Face)	60' - 5'	1	No Ice 0.00 1/2" Ice 0.00 1" Ice 0.00	0.00

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A _R	A _F	C _{A,A} In Face	C _{A,A} Out Face	Weight
	ft		ft ²	ft ²	ft ²	ft ²	K
L1	170'-150'	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.29
L2	150'-126'3-3/8"	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.94
L3	126'3-3/8"-94'11-1	A	0.000	0.000	0.000	0.000	0.00

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Tower Section	Tower Elevation	Face	A_R	A_F	$C_A A_A$ In Face	$C_A A_A$ Out Face	Weight
			ft ²	ft ²	ft ²	ft ²	K
L4	94'11"-13/32"-63'8"- 5/8"	B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.95
		A	0.000	0.000	0.000	0.000	0.00
L5	63'8"-5/8"-31'9"-15/ 32"	B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.96
		A	0.000	0.000	0.000	0.000	0.00
L6	31'9"-15/32"-1"	B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	2.04
		A	0.000	0.000	0.000	0.000	0.00

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face or Leg	Ice Thickness	A_R	A_F	$C_A A_A$ In Face	$C_A A_A$ Out Face	Weight
			in	ft ²	ft ²	ft ²	ft ²	K
L1	170'-150'	A	1.757	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	2.96
L2	150'-126'3"-3/8"	A	1.730	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	9.32
L3	126'3"-3/8"-94'11"-1 3/32"	A	1.692	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	19.38
L4	94'11"-13/32"-63'8"- 5/8"	A	1.637	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	18.93
L5	63'8"-5/8"-31'9"-15/ 32"	A	1.556	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	18.96
L6	31'9"-15/32"-1"	A	1.399	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	14.98

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
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Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	$C_A A_A$ Front	$C_A A_A$ Side	Weight	
			ft	°	ft	ft ²	ft ²	K	
21.9' Omni Antenna	C	From Leg	1.50	0.00	170'	No Ice	3.00	3.00	0.01

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
21.9' Omni Antenna	B	From Leg	0' 0' 1.50 0' 0'	0.00	170'	1/2" Ice 0.00 1" Ice 0.00 No Ice 3.00 1/2" Ice 0.00 1" Ice 0.00	0.00 0.00 3.00 0.00 0.00	0.02 0.03 0.01 0.02 0.03
21.9' Omni Antenna	A	From Leg	1.50 0' 0'	0.00	170'	No Ice 3.00 1/2" Ice 0.00 1" Ice 0.00	3.00 0.00 0.00	0.01 0.02 0.01
*****						1/2" Ice 0.00 1" Ice 0.00	0.00 0.00	0.03
12.6' Low Profile Platform w/Rails Sector	A	From Face	2.00 0' 0'	0.00	166'	No Ice 11.00 1/2" Ice 0.00 1" Ice 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	A	From Face	3.00 0' 4'	0.00	166'	No Ice 11.47 1/2" Ice 12.08 1" Ice 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(4) Ericsson RRUS 11	A	From Face	2.00 0' 4'	0.00	166'	No Ice 2.80 1/2" Ice 3.01 1" Ice 3.22	1.57 1.83 2.11	0.06 0.09 0.12
(2) Raycap DC6-48-60-18-8F	A	From Face	0.00 0' 4'	0.00	166'	No Ice 2.22 1/2" Ice 2.44 1" Ice 2.66	2.22 2.44 2.66	0.02 0.04 0.06
(2) Ericsson RRUS A2 Module	A	From Face	2.00 0' 4'	0.00	166'	No Ice 2.06 1/2" Ice 2.24 1" Ice 2.43	0.75 0.95 1.17	0.03 0.04 0.07
12.6' Low Profile Platform w/Rails Sector	B	From Face	2.00 0' 0'	0.00	166'	No Ice 11.00 1/2" Ice 0.00 1" Ice 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	B	From Face	3.00 0' 4'	0.00	166'	No Ice 11.47 1/2" Ice 12.08 1" Ice 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(4) Ericsson RRUS 11	B	From Face	2.00 0' 4'	0.00	166'	No Ice 2.80 1/2" Ice 3.01 1" Ice 3.22	1.57 1.83 2.11	0.06 0.09 0.12
Raycap DC6-48-60-18-8F	B	From Face	0.00 0' 4'	0.00	166'	No Ice 2.22 1/2" Ice 2.44 1" Ice 2.66	2.22 2.44 2.66	0.02 0.04 0.06
(2) Ericsson RRUS A2 Module	B	From Face	2.00 0' 4'	0.00	166'	No Ice 2.06 1/2" Ice 2.24 1" Ice 2.43	0.75 0.95 1.17	0.03 0.04 0.07
12.6' Low Profile Platform w/Rails Sector	C	From Face	2.00 0' 0'	0.00	166'	No Ice 11.00 1/2" Ice 0.00 1" Ice 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	C	From Face	3.00 0' 4'	0.00	166'	No Ice 11.47 1/2" Ice 12.08 1" Ice 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(4) Ericsson RRUS 11	C	From Face	2.00 0' 4'	0.00	166'	No Ice 2.80 1/2" Ice 3.01 1" Ice 3.22	1.57 1.83 2.11	0.06 0.09 0.12
Raycap DC6-48-60-18-8F	C	From Face	0.00 0' 4'	0.00	166'	No Ice 2.22 1/2" Ice 2.44 1" Ice 2.66	2.22 2.44 2.66	0.02 0.04 0.06
(2) Ericsson RRUS A2 Module	C	From Face	2.00 0' 4'	0.00	166'	No Ice 2.06 1/2" Ice 2.24 1" Ice 2.43	0.75 0.95 1.17	0.03 0.04 0.07
*****						No Ice 11.00 1/2" Ice 0.00 1" Ice 0.00	11.00 0.00 0.00	0.50 0.65 0.80
12.6' Low Profile Platform w/Rails Sector	A	From Face	2.00 0'	0.00	156'	No Ice 11.00 1/2" Ice 0.00	11.00 0.00	0.50 0.65

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C�A _A	C�A _A	Weight K	
						Front	Side		
(4) Generic 96"x12"x7" Panel	A	From Face	3.00 0' 0'	0.00	156'	1" Ice No Ice 1/2" Ice 1" Ice 1" Ice	0.00 11.47 12.08 12.71 3.22	0.00 9.48 10.90 12.17 2.11	0.80 0.07 0.16 0.25 0.12
(4) Ericsson RRUS 11	A	From Face	2.00 0' 0'	0.00	156'	No Ice 1/2" Ice 1" Ice 2.80 3.01 3.22	2.80 1.57 1.83 11.00 12.08 2.44	1.57 0.06 0.09 0.25 0.12	0.06
Raycap DC6-48-60-18-8F	A	None		0.00	156'	No Ice 1/2" Ice 1" Ice	2.22 2.44 2.66	2.22 2.44 2.66	0.02 0.04 0.06
12.6' Low Profile Platform w/Rails Sector	B	From Face	2.00 0' -3'	0.00	156'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	B	From Face	3.00 0' 0'	0.00	156'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(4) Ericsson RRUS 11	B	From Face	2.00 0' 0'	0.00	156'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11	0.06 0.09 0.12
12.6' Low Profile Platform w/Rails Sector	C	From Face	2.00 0' -3'	0.00	156'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	C	From Face	3.00 0' 0'	0.00	156'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(4) Ericsson RRUS 11	C	From Face	2.00 0' 0'	0.00	156'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11	0.06 0.09 0.12

12.6' Low Profile Platform w/Rails Sector	A	From Face	2.00 0' -3'	0.00	146'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	A	From Face	3.00 0' 0'	0.00	146'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(8) Ericsson RRUS 11	A	From Face	2.00 0' 0'	0.00	146'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11	0.06 0.09 0.12
(2) Raycap DC6-48-60-18-8F	A	None		0.00	146'	No Ice 1/2" Ice 1" Ice	2.22 2.44 2.66	2.22 2.44 2.66	0.02 0.04 0.06
(2) Ericsson RRUS A2 Module	A	From Face	2.00 0' 0'	0.00	146'	No Ice 1/2" Ice 1" Ice	2.06 2.24 2.43	0.75 0.95 1.17	0.03 0.04 0.07
12.6' Low Profile Platform w/Rails Sector	B	From Face	2.00 0' -3'	0.00	146'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.00	0.50 0.65 0.80
(4) Generic 96"x12"x7" Panel	B	From Face	3.00 0' 0'	0.00	146'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17	0.07 0.16 0.25
(8) Ericsson RRUS 11	B	From Face	2.00 0' 0'	0.00	146'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11	0.06 0.09 0.12
Raycap DC6-48-60-18-8F	B	None		0.00	146'	No Ice 1/2" Ice 1" Ice	2.22 2.44 2.66	2.22 2.44 2.66	0.02 0.04 0.06
(2) Ericsson RRUS A2	B	From Face	2.00	0.00	146'	No Ice	2.06	0.75	0.03

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Description		Face or Leg	Offset Type	Offsets: Lateral	Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight	
				Horz ft	Vert ft	°	ft	ft ²	ft ²	K
Module				0'			1/2" Ice	2.24	0.95	0.04
				0'			1" Ice	2.43	1.17	0.07
12.6' Low Profile Platform w/Rails Sector	C	From Face	2.00	0.00	146'		No Ice	11.00	11.00	0.50
			0'				1/2" Ice	0.00	0.00	0.65
			-3'				1" Ice	0.00	0.00	0.80
(4) Generic 96"x12"x7" Panel	C	From Face	3.00	0.00	146'		No Ice	11.47	9.48	0.07
			0'				1/2" Ice	12.08	10.90	0.16
			0'				1" Ice	12.71	12.17	0.25
(8) Ericsson RRUS 11	C	From Face	2.00	0.00	146'		No Ice	2.80	1.57	0.06
			0'				1/2" Ice	3.01	1.83	0.09
			0'				1" Ice	3.22	2.11	0.12
Raycap DC6-48-60-18-8F	C	None		0.00	146'		No Ice	2.22	2.22	0.02
							1/2" Ice	2.44	2.44	0.04
							1" Ice	2.66	2.66	0.06
(2) Ericsson RRUS A2 Module	C	From Face	2.00	0.00	146'		No Ice	2.06	0.75	0.03
			0'				1/2" Ice	2.24	0.95	0.04
			0'				1" Ice	2.43	1.17	0.07

12.6' Low Profile Platform w/Rails Sector	A	From Face	2.00	0.00	136'		No Ice	11.00	11.00	0.50
			0'				1/2" Ice	0.00	0.00	0.65
			-3'				1" Ice	0.00	0.00	0.80
(4) Generic 96"x12"x7" Panel	A	From Face	3.00	0.00	136'		No Ice	11.47	9.48	0.07
			0'				1/2" Ice	12.08	10.90	0.16
			0'				1" Ice	12.71	12.17	0.25
(4) Ericsson RRUS 11	A	From Face	2.00	0.00	136'		No Ice	2.80	1.57	0.06
			0'				1/2" Ice	3.01	1.83	0.09
			0'				1" Ice	3.22	2.11	0.12
Raycap DC6-48-60-18-8F	A	None		0.00	136'		No Ice	2.22	2.22	0.02
							1/2" Ice	2.44	2.44	0.04
							1" Ice	2.66	2.66	0.06
(2) PCTEL GPS-TMG-HR-26N 5"x3.2"	A	None		0.00	136'		No Ice	0.08	0.08	0.01
							1/2" Ice	0.12	0.12	0.01
							1" Ice	0.18	0.18	0.01
(2) Dplexers 5.8x6.5x1.5 FD9R6004/1C-3L	A	From Face	2.00	0.00	136'		No Ice	0.31	0.09	0.03
			0'				1/2" Ice	0.39	0.13	0.03
			0'				1" Ice	0.47	0.19	0.04
12.6' Low Profile Platform w/Rails Sector	B	From Face	2.00	0.00	136'		No Ice	11.00	11.00	0.50
			0'				1/2" Ice	0.00	0.00	0.65
			-3'				1" Ice	0.00	0.00	0.80
(4) Generic 96"x12"x7" Panel	B	From Face	3.00	0.00	136'		No Ice	11.47	9.48	0.07
			0'				1/2" Ice	12.08	10.90	0.16
			0'				1" Ice	12.71	12.17	0.25
(4) Ericsson RRUS 11	B	From Face	2.00	0.00	136'		No Ice	2.80	1.57	0.06
			0'				1/2" Ice	3.01	1.83	0.09
			0'				1" Ice	3.22	2.11	0.12
Raycap DC6-48-60-18-8F	B	None		0.00	136'		No Ice	2.22	2.22	0.02
							1/2" Ice	2.44	2.44	0.04
							1" Ice	2.66	2.66	0.06
PCTEL GPS-TMG-HR-26N 5"x3.2"	B	None		0.00	136'		No Ice	0.08	0.08	0.01
							1/2" Ice	0.12	0.12	0.01
							1" Ice	0.18	0.18	0.01
(2) Dplexers 5.8x6.5x1.5 FD9R6004/1C-3L	B	From Face	2.00	0.00	136'		No Ice	0.31	0.09	0.03
			0'				1/2" Ice	0.39	0.13	0.03
			0'				1" Ice	0.47	0.19	0.04
12.6' Low Profile Platform w/Rails Sector	C	From Face	2.00	0.00	136'		No Ice	11.00	11.00	0.50
			0'				1/2" Ice	0.00	0.00	0.65
			2'				1" Ice	0.00	0.00	0.80

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _I Front	C _A A _I Side	Weight K
(4) Generic 96"x12"x7" Panel	C	From Face	3.00 0' 0'	0.00	136'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17
(4) Ericsson RRUS 11	C	From Face	2.00 0' 0'	0.00	136'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11
Raycap DC6-48-60-18-8F	C	None		0.00	136'	No Ice 1/2" Ice 1" Ice	2.22 2.44 2.66	0.02 0.04 0.06
PCTEL GPS-TMG-HR-26N 5"x3.2"	C	None		0.00	136'	No Ice 1/2" Ice 1" Ice	0.08 0.12 0.18	0.01 0.01 0.01
(2) Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	C	From Face	2.00 0' 0'	0.00	136'	No Ice 1/2" Ice 1" Ice	0.31 0.39 0.47	0.03 0.03 0.04

12.6' Low Profile Platform w/Rails Sector	A	From Face	2.00 0' -3'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.80
(4) Generic 96"x12"x7" Panel	A	From Face	3.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17
(4) Ericsson RRUS 11	A	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11
Raycap DC6-48-60-18-8F	A	None		0.00	126'	No Ice 1/2" Ice 1" Ice	2.22 2.44 2.66	0.02 0.04 0.06
(4) E15S09P78 (TMA)	A	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	0.75 0.86 0.97	0.53 0.68 0.85
12.6' Low Profile Platform w/Rails Sector	B	From Face	2.00 0' -3'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.80
(4) Generic 96"x12"x7" Panel	B	From Face	3.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17
(4) Ericsson RRUS 11	B	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11
(4) E15S09P78 (TMA)	B	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	0.75 0.86 0.97	0.53 0.68 0.85
12.6' Low Profile Platform w/Rails Sector	C	From Face	2.00 0' -3'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.00 0.00 0.00	11.00 0.00 0.80
(4) Generic 96"x12"x7" Panel	C	From Face	3.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	11.47 12.08 12.71	9.48 10.90 12.17
(4) Ericsson RRUS 11	C	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	2.80 3.01 3.22	1.57 1.83 2.11
(4) E15S09P78 (TMA)	C	From Face	2.00 0' 0'	0.00	126'	No Ice 1/2" Ice 1" Ice	0.75 0.86 0.97	0.53 0.68 0.85

21.6' C	G	From Face	1.50 0' 0'	0.00	75'	No Ice	3.00	3.00

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	$C_A A_{\text{Front}}$	$C_A A_{\text{Side}}$	Weight
			ft	°	ft	ft ²	ft ²	K
			ft					
			ft					
			ft					
			0'			1/2" Ice	0.00	0.00
			0'			1" Ice	0.00	0.00
								0.02
								0.03

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight
				ft	°	°	ft	ft	ft ²	K
1' HP		Paraboloid w/Shroud (HP)	None		0.00		170'	1.00	No Ice	0.79
									1/2" Ice	0.92
									1" Ice	1.06
2ft Dish		Paraboloid w/Shroud (HP)	None		0.00		60'	2.00	No Ice	3.14
									1/2" Ice	3.41
									1" Ice	3.68
										0.05
										0.07

Tower Pressures - No Ice

$$G_H = 1.100$$

Section Elevation	z	K_z	q_z	A_G	F_a	A_F	A_R	A_{leg}	Leg %	$C_A A_{\text{In Face}}$	$C_A A_{\text{Out Face}}$
	ft	ft	ksf	ft ²	%	ft ²	ft ²				
L1 170'-150'	160'	1.397	0	43.975	A	0.000	43.975	43.975	100.00	0.000	0.000
					B	0.000	43.975		100.00	0.000	0.000
					C	0.000	43.975		100.00	0.000	0.000
L2 150'-126'3-3/8"	137'7-29/3 2"	1.354	0	59.377	A	0.000	59.377	59.377	100.00	0.000	0.000
					B	0.000	59.377		100.00	0.000	0.000
					C	0.000	59.377		100.00	0.000	0.000
L3 126'3-3/8"-94'1 1-13/32"	110'27/32"	1.291	0	99.311	A	0.000	99.311	99.311	100.00	0.000	0.000
					B	0.000	99.311		100.00	0.000	0.000
					C	0.000	99.311		100.00	0.000	0.000
L4 94'11-13/32"-6 3'8-5/8"	78'11-17/3 2"	1.204	0	122.411	A	0.000	122.411	122.411	100.00	0.000	0.000
					B	0.000	122.411		100.00	0.000	0.000
					C	0.000	122.411		100.00	0.000	0.000
L5 63'8-5/8"-31'9- 15/32"	476-27/32'	1.082	0	148.858	A	0.000	148.858	148.858	100.00	0.000	0.000
					B	0.000	148.858		100.00	0.000	0.000
					C	0.000	148.858		100.00	0.000	0.000
L6 31'9-15/32"-1'	16'4-13/16'	0.865	0	166.335	A	0.000	166.335	166.335	100.00	0.000	0.000
					B	0.000	166.335		100.00	0.000	0.000
					C	0.000	166.335		100.00	0.000	0.000

Tower Pressure - With Ice

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$G_H = 1.100$

Section Elevation	z ft	K_z	q_z	t_2	A_G	F_a c e	A_F	A_R	A_{leg}	Leg %	$C_A A_d$ In Face ft^2	$C_A A_d$ Out Face ft^2
	ft	ft	ksf	in	ft ²		ft ²	ft ²	ft ²			
L1 170'-150'	160'	1.397	0	1.76	49.830	A B C	0.000 49.830 49.830	49.830	49.830	100.00	0.000	0.000
L2 150'-126'3-3/8"	137'7-29/32'	1.354	0	1.73	66.217	A B C	0.000 66.217 66.217	66.217	66.217	100.00	0.000	0.000
L3 126'3-3/8"-94'11"-13/32"	110'27/32"	1.291	0	1.69	108.346	A B C	0.000 108.346 108.346	108.346	108.346	100.00	0.000	0.000
L4 94'11-13/32"-63'8-5/8"	78'11-17/32'	1.204	0	1.64	131.218	A B C	0.000 131.218 131.218	131.218	131.218	100.00	0.000	0.000
L5 63'8-5/8"-31'9-1-5/32"	476-27/32"	1.082	0	1.56	157.566	A B C	0.000 157.566 157.566	157.566	157.566	100.00	0.000	0.000
L6 31'9-15/32"-1"	16'4-13/16"	0.865	0	1.40	174.320	A B C	0.000 174.320 174.320	174.320	174.320	100.00	0.000	0.000

Tower Pressure - Service

$G_H = 1.100$

Section Elevation	z ft	K_z	q_z	A_G	F_a c e	A_F	A_R	A_{leg}	Leg %	$C_A A_d$ In Face ft^2	$C_A A_d$ Out Face ft^2
	ft	ft	ksf	ft ²		ft ²	ft ²	ft ²			
L1 170'-150'	160'	1.397	0	43.975	A B C	0.000 43.975 43.975	43.975	43.975	100.00	0.000	0.000
L2 150'-126'3-3/8"	137'7-29/32"	1.354	0	59.377	A B C	0.000 59.377 59.377	59.377	59.377	100.00	0.000	0.000
L3 126'3-3/8"-94'11"-13/32"	110'27/32"	1.291	0	99.311	A B C	0.000 99.311 99.311	99.311	99.311	100.00	0.000	0.000
L4 94'11-13/32"-63'8-5/8"	78'11-17/32"	1.204	0	122.411	A B C	0.000 122.411 122.411	122.411	122.411	100.00	0.000	0.000
L5 63'8-5/8"-31'9-1-15/32"	476-27/32"	1.082	0	148.858	A B C	0.000 148.858 148.858	148.858	148.858	100.00	0.000	0.000
L6 31'9-15/32"-1"	16'4-13/16"	0.865	0	166.335	A B C	0.000 166.335 166.335	166.335	166.335	100.00	0.000	0.000

Tower Forces - No Ice - Wind Normal To Face

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _r	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.87	0.04	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	1.13	0.05	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94' 11-13/32"	1.95	4.47	A	1	0.65	0	1	1	99.311	1.81	0.06	C
			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"- 63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	2.07	0.07	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9" -15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	2.26	0.07	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1'	1.71	11.26	A	1	0.65	0	1	1	166.335	2.09	0.07	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		788.36 kip-ft	10.23		

Tower Forces - No Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _r	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.87	0.04	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	1.13	0.05	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94' 11-13/32"	1.95	4.47	A	1	0.65	0	1	1	99.311	1.81	0.06	C
			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"- 63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	2.07	0.07	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9" -15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	2.26	0.07	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1'	1.71	11.26	A	1	0.65	0	1	1	166.335	2.09	0.07	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		788.36 kip-ft	10.23		

Tower Forces - No Ice - Wind 90 To Face

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _r ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.87	0.04	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	1.13	0.05	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94' 11-13/32"	1.95	4.47	A	1	0.65	0	1	1	99.311	1.81	0.06	C
			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"- 63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	2.07	0.07	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9" -15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	2.26	0.07	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1'	1.71	11.26	A	1	0.65	0	1	1	166.335	2.09	0.07	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		788.36 kip-ft	10.23		

Tower Forces - With Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _r ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	2.96	2.30	A	1	1.2	0	1	1	49.830	0.56	0.03	C
			B	1	1.2		1	1	49.830			
			C	1	1.2		1	1	49.830			
L2 150'-126'3-3/8"	9.32	3.36	A	1	1.2	0	1	1	66.217	0.72	0.03	C
			B	1	1.2		1	1	66.217			
			C	1	1.2		1	1	66.217			
L3 126'3-3/8"-94' 11-13/32"	19.38	7.03	A	1	1.2	0	1	1	108.346	1.12	0.04	C
			B	1	1.2		1	1	108.346			
			C	1	1.2		1	1	108.346			
L4 94'11-13/32"- 63'8-5/8"	18.93	10.13	A	1	1.2	0	1	1	131.218	1.27	0.04	C
			B	1	1.2		1	1	131.218			
			C	1	1.2		1	1	131.218			
L5 63'8-5/8"-31'9" -15/32"	18.96	12.31	A	1	1.2	0	1	1	157.566	1.36	0.04	C
			B	1	1.2		1	1	157.566			
			C	1	1.2		1	1	157.566			
L6 31'9-15/32"-1'	14.98	14.73	A	1	1.2	0	1	1	174.320	1.25	0.04	C
			B	1	1.2		1	1	174.320			
			C	1	1.2		1	1	174.320			
Sum Weight:	84.53	49.86					OTM		491.03 kip-ft	6.28		

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job 150ft of 170ft.90mph							Page 13 of 33		
	Project Brookfield CT							Date 17:04:39 01/11/17		
	Client Ambar							Designed by Chunhui Song		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	2.96	2.30	A	1	1.2	0	1	1	49.830	0.56	0.03	C
			B	1	1.2		1	1	49.830			
			C	1	1.2		1	1	49.830			
L2 150'-126'3-3/8"	9.32	3.36	A	1	1.2	0	1	1	66.217	0.72	0.03	C
			B	1	1.2		1	1	66.217			
			C	1	1.2		1	1	66.217			
L3 126'3-3/8"-94"	19.38	7.03	A	1	1.2	0	1	1	108.346	1.12	0.04	C
			B	1	1.2		1	1	108.346			
			C	1	1.2		1	1	108.346			
L4 94'11-13/32"-63'8-5/8"	18.93	10.13	A	1	1.2	0	1	1	131.218	1.27	0.04	C
			B	1	1.2		1	1	131.218			
			C	1	1.2		1	1	131.218			
L5 63'8-5/8"-31'9"-15/32"	18.96	12.31	A	1	1.2	0	1	1	157.566	1.36	0.04	C
			B	1	1.2		1	1	157.566			
			C	1	1.2		1	1	157.566			
L6 31'9-15/32"-1"	14.98	14.73	A	1	1.2	0	1	1	174.320	1.25	0.04	C
			B	1	1.2		1	1	174.320			
			C	1	1.2		1	1	174.320			
Sum Weight:	84.53	49.86					OTM		491.03 kip-ft	6.28		

Tower Forces - With Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	2.96	2.30	A	1	1.2	0	1	1	49.830	0.56	0.03	C
			B	1	1.2		1	1	49.830			
			C	1	1.2		1	1	49.830			
L2 150'-126'3-3/8"	9.32	3.36	A	1	1.2	0	1	1	66.217	0.72	0.03	C
			B	1	1.2		1	1	66.217			
			C	1	1.2		1	1	66.217			
L3 126'3-3/8"-94"	19.38	7.03	A	1	1.2	0	1	1	108.346	1.12	0.04	C
			B	1	1.2		1	1	108.346			
			C	1	1.2		1	1	108.346			
L4 94'11-13/32"-63'8-5/8"	18.93	10.13	A	1	1.2	0	1	1	131.218	1.27	0.04	C
			B	1	1.2		1	1	131.218			
			C	1	1.2		1	1	131.218			
L5 63'8-5/8"-31'9"-15/32"	18.96	12.31	A	1	1.2	0	1	1	157.566	1.36	0.04	C
			B	1	1.2		1	1	157.566			
			C	1	1.2		1	1	157.566			
L6 31'9-15/32"-1"	14.98	14.73	A	1	1.2	0	1	1	174.320	1.25	0.04	C
			B	1	1.2		1	1	174.320			
			C	1	1.2		1	1	174.320			
Sum Weight:	84.53	49.86					OTM		491.03 kip-ft	6.28		

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job 150ft of 170ft.90mph								Page 14 of 33		
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	Client Ambor								Designed by Chunhui Song		

Tower Forces - Service - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.34	0.02	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	0.45	0.02	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94"	1.95	4.47	A	1	0.65	0	1	1	99.311	0.72	0.02	C
11-13/32"			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"-63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	0.82	0.03	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9"-15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	0.90	0.03	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1"	1.71	11.26	A	1	0.65	0	1	1	166.335	0.83	0.03	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		313.50 kip-ft	4.07		

Tower Forces - Service - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E	F	w	Ctrl. Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.34	0.02	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	0.45	0.02	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94"	1.95	4.47	A	1	0.65	0	1	1	99.311	0.72	0.02	C
11-13/32"			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"-63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	0.82	0.03	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9"-15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	0.90	0.03	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1"	1.71	11.26	A	1	0.65	0	1	1	166.335	0.83	0.03	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		313.50 kip-ft	4.07		

tnxTower Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job 150ft of 170ft.90mph							Page 15 of 33	
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	Client Ambor							Designed by Chunhui Song	

Tower Forces - Service - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F	w klf	Ctrl Face
L1 170'-150'	0.29	1.10	A	1	0.65	0	1	1	43.975	0.34	0.02	C
			B	1	0.65		1	1	43.975			
			C	1	0.65		1	1	43.975			
L2 150'-126'3-3/8"	0.94	1.78	A	1	0.65	0	1	1	59.377	0.45	0.02	C
			B	1	0.65		1	1	59.377			
			C	1	0.65		1	1	59.377			
L3 126'3-3/8"-94"	1.95	4.47	A	1	0.65	0	1	1	99.311	0.72	0.02	C
			B	1	0.65		1	1	99.311			
			C	1	0.65		1	1	99.311			
L4 94'11-13/32"-63'8-5/8"	1.96	7.10	A	1	0.65	0	1	1	122.411	0.82	0.03	C
			B	1	0.65		1	1	122.411			
			C	1	0.65		1	1	122.411			
L5 63'8-5/8"-31'9"-15/32"	2.04	8.84	A	1	0.65	0	1	1	148.858	0.90	0.03	C
			B	1	0.65		1	1	148.858			
			C	1	0.65		1	1	148.858			
L6 31'9-15/32"-1'	1.71	11.26	A	1	0.65	0	1	1	166.335	0.83	0.03	C
			B	1	0.65		1	1	166.335			
			C	1	0.65		1	1	166.335			
Sum Weight:	8.89	34.55					OTM		313.50 kip-ft	4.07		

Discrete Appurtenance Pressures - No Ice G_H = 1.100

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _z ksf	C _{AAC} Front ft ²	C _{AAC} Side ft ²
21.9' Omni Antenna	240.00	0.01	-2'2-7/8"	1'3-15/32"	170'	1.415	0	3.00	3.00
21.9' Omni Antenna	120.00	0.01	2'2-7/8"	1'3-15/32"	170'	1.415	0	3.00	3.00
21.9' Omni Antenna	0.00	0.01	0'	-2'6-31/32"	170'	1.415	0	3.00	3.00
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"	300.00	0.50	-2'8-1/32"	-1'6-15/32"	166'	1.408	0	11.00	11.00
Panel Ericsson RRUS 11	300.00	0.28	-3'6-15/32"	-2'15/32"	170'	1.415	0	45.87	37.91
Raycap DC6-48-60-18-8F Ericsson RRUS A2 Module	300.00	0.24	-2'8-1/32"	-1'6-15/32"	170'	1.415	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"	60.00	0.50	2'8-1/32"	-1'6-15/32"	166'	1.408	0	11.00	11.00
Panel Ericsson RRUS 11	60.00	0.28	3'6-15/32"	-2'15/32"	170'	1.415	0	45.87	37.91
Raycap	60.00	0.24	2'8-1/32"	-1'6-15/32"	170'	1.415	0	11.19	6.27
Ericsson RRUS A2 Module	60.00	0.02	11-9/32"	-6-15/32"	170'	1.415	0	2.22	2.22

<i>tnxTower</i> Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job 150ft of 170ft.90mph	Page 16 of 33
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	Client Ambar	Designed by Chunhui Song

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _x ksf	C _{Ac} Front ft ²	C _{Ac} Side ft ²
DC6-48-60-18-8F Ericsson RRUS A2 Module	60.00	0.06	2'8-1/32"	-1'6-15/32"	170'	1.415	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0'	3'31/32"	166'	1.408	0	11.00	11.00
Ericsson RRUS 11 Raycap	180.00	0.24	0'	4'31/32"	170'	1.415	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS A2 Module	180.00	0.06	0'	3'31/32"	170'	1.415	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.50	-2'8-1/32"	-1'6-15/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11 Raycap	300.00	0.28	-3'6-15/32"	-2'15/32"	156'	1.390	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS 11 Raycap	300.00	0.24	-2'8-1/32"	-1'6-15/32"	156'	1.390	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'8-1/32"	-1'6-15/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11 Raycap	60.00	0.28	3'6-15/32"	-2'15/32"	156'	1.390	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS 11 Raycap	60.00	0.24	2'8-1/32"	-1'6-15/32"	156'	1.390	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0'	3'31/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11 Raycap	180.00	0.28	0'	4'31/32"	156'	1.390	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS 11 Raycap	180.00	0.24	0'	3'31/32"	156'	1.390	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.50	-2'8-17/32"	-1'6-27/32"	143'	1.365	0	11.00	11.00
Ericsson RRUS 11 Raycap	300.00	0.28	-3'6-31/32"	-2'27/32"	146'	1.371	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS 11 Raycap	300.00	0.48	-2'8-17/32"	-1'6-27/32"	146'	1.371	0	22.37	12.54
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.48	2'8-17/32"	-1'6-27/32"	146'	1.371	0	4.44	4.44
Ericsson RRUS 11 Raycap	60.00	0.06	-2'8-17/32"	-1'6-27/32"	146'	1.371	0	4.13	1.50
DC6-48-60-18-8F Ericsson RRUS 11 Raycap	60.00	0.50	2'8-17/32"	-1'6-27/32"	143'	1.365	0	11.00	11.00
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.28	3'6-31/32"	-2'27/32"	146'	1.371	0	45.87	37.91
Ericsson RRUS 11 Raycap	60.00	0.48	2'8-17/32"	-1'6-27/32"	146'	1.371	0	22.37	12.54
DC6-48-60-18-8F Ericsson RRUS A2 Raycap	60.00	0.02	0'	0"	146'	1.371	0	2.22	2.22
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0'	3'1-9/16"	143'	1.365	0	11.00	11.00
Ericsson RRUS 11 Raycap	180.00	0.28	0'	4'1-9/16"	146'	1.371	0	45.87	37.91
DC6-48-60-18-8F Ericsson RRUS A2 Raycap	180.00	0.48	0'	3'1-9/16"	146'	1.371	0	22.37	12.54
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	0.00	0.02	0'	0"	146'	1.371	0	2.22	2.22
Ericsson RRUS 11 Raycap	180.00	0.06	0'	3'1-9/16"	146'	1.371	0	4.13	1.50
DC6-48-60-18-8F Ericsson RRUS A2 Raycap	180.00	0.50	-2'9-27/32"	-1'7-9/16"	133'	1.344	0	11.00	11.00

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job 150ft of 170ft.90mph						Page 17 of 33	
	Project Brookfield CT						Date 17:04:39 01/11/17	
	Client Ambor						Designed by Chunhui Song	

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _z ksf	C _{AAC} Front ft ²	C _{AAC} Side ft ²
Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.28	-3'8-9/32"	-2'1-9/16"	136'	1.350	0	45.87	37.91
Ericsson RRUS 11	300.00	0.24	-2'9-27/32"	-1'7-9/16"	136'	1.350	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	136'	1.350	0	2.22	2.22
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.02	0'	0'	136'	1.350	0	0.16	0.16
Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	300.00	0.06	-2'9-27/32"	-1'7-9/16"	136'	1.350	0	0.63	0.17
12.6' Low Profile	60.00	0.50	2'9-27/32"	-1'7-9/16"	133'	1.344	0	11.00	11.00
Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.28	3'8-9/32"	-2'1-9/16"	136'	1.350	0	45.87	37.91
Ericsson RRUS 11	60.00	0.24	2'9-27/32"	-1'7-9/16"	136'	1.350	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	136'	1.350	0	2.22	2.22
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.01	0'	0'	136'	1.350	0	0.08	0.08
Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	60.00	0.06	2'9-27/32"	-1'7-9/16"	136'	1.350	0	0.63	0.17
12.6' Low Profile	180.00	0.50	0'	3'3-1/8"	133'	1.344	0	11.00	11.00
Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.28	0'	4'3-1/8"	136'	1.350	0	45.87	37.91
Ericsson RRUS 11	180.00	0.24	0'	3'3-1/8"	136'	1.350	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	136'	1.350	0	2.22	2.22
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.01	0'	0'	136'	1.350	0	0.08	0.08
Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	180.00	0.06	0'	3'3-1/8"	136'	1.350	0	0.63	0.17
12.6' Low Profile	300.00	0.50	-2'11-1/32"	-1'8-5/32"	123'	1.322	0	11.00	11.00
Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.28	-3'9-3/8"	-2'2-5/32"	126'	1.329	0	45.87	37.91
Ericsson RRUS 11	300.00	0.24	-2'11-1/32"	-1'8-5/32"	126'	1.329	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	126'	1.329	0	2.22	2.22
E15S09P78 (TMA)	300.00	0.08	-2'11-1/32"	-1'8-5/32"	126'	1.329	0	2.99	2.10
12.6' Low Profile	60.00	0.50	2'11-1/32"	-1'8-5/32"	123'	1.322	0	11.00	11.00
Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.28	3'9-3/8"	-2'2-5/32"	126'	1.329	0	45.87	37.91
Ericsson RRUS 11	60.00	0.24	2'11-1/32"	-1'8-5/32"	126'	1.329	0	11.19	6.27
E15S09P78 (TMA)	60.00	0.08	2'11-1/32"	-1'8-5/32"	126'	1.329	0	2.99	2.10
12.6' Low Profile	180.00	0.50	0'	3'4-7/16"	123'	1.322	0	11.00	11.00
Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.28	0'	4'4-7/16"	126'	1.329	0	45.87	37.91
Ericsson RRUS 11	180.00	0.24	0'	3'4-7/16"	126'	1.329	0	11.19	6.27
E15S09P78 (TMA)	180.00	0.08	0'	3'4-7/16"	126'	1.329	0	2.99	2.10

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	Project Brookfield CT							Date 17:04:39 01/11/17
	Client Ambar							Designed by Chunhui Song

Description	Aiming Azimuth °	Weight	Offset _x	Offset _z	z	K _z	q _z	C _{AAC} Front ft ²	C _{AAC} Side ft ²
		K	ft	ft	ft		ksf		
21.9' Omni		180.00	0.01	0'	3'5-3/4"	75'	1.191	0	3.00
		Sum Weight:	17.15						3.00

Discrete Appurtenance Pressures - With Ice G_H = 1.100

Description	Aiming Azimuth °	Weight	Offset _x	Offset _z	z	K _z	q _z	C _{AAC} Front ft ²	C _{AAC} Side ft ²	t _x
		K	ft	ft	ft		ksf			in
21.9' Omni Antenna		240.00	0.04	-2'2-7/8"	1'3-15/32"	170'	1.415	0	0.00	0.00
21.9' Omni Antenna		120.00	0.04	2'2-7/8"	1'3-15/32"	170'	1.415	0	0.00	0.00
21.9' Omni Antenna		0.00	0.04	0'	-2'6-31/32"	170'	1.415	0	0.00	0.00
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"		300.00	1.03	-2'8-1/32"	-1'6-15/32"	166'	1.408	0	0.00	0.00
Panel		300.00	1.70	-3'6-15/32"	-2'15/32"	170'	1.415	0	54.61	55.41
Ericsson RRUS 11		300.00	0.72	-2'8-1/32"	-1'6-15/32"	170'	1.415	0	14.28	10.34
Raycap DC6-48-60-18-8F		300.00	0.18	-11-9/32"	-6-15/32"	170'	1.415	0	5.99	5.99
Ericsson RRUS A2 Module		300.00	0.21	-2'8-1/32"	-1'6-15/32"	170'	1.415	0	5.46	3.10
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"		60.00	1.03	2'8-1/32"	-1'6-15/32"	166'	1.408	0	0.00	0.00
Panel		60.00	1.70	3'6-15/32"	-2'15/32"	170'	1.415	0	54.61	55.41
Ericsson RRUS 11		60.00	0.72	2'8-1/32"	-1'6-15/32"	170'	1.415	0	14.28	10.34
Raycap DC6-48-60-18-8F		60.00	0.09	11-9/32"	-6-15/32"	170'	1.415	0	3.00	3.00
Ericsson RRUS A2 Module		60.00	0.21	2'8-1/32"	-1'6-15/32"	170'	1.415	0	5.46	3.10
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"		180.00	1.03	0'	3'31/32"	166'	1.408	0	0.00	0.00
Panel		180.00	1.70	0'	4'31/32"	170'	1.415	0	54.61	55.41
Ericsson RRUS 11		180.00	0.72	0'	3'31/32"	170'	1.415	0	14.28	10.34
Raycap DC6-48-60-18-8F		180.00	0.09	0'	1'31/32"	170'	1.415	0	3.00	3.00
Ericsson RRUS A2 Module		180.00	0.21	0'	3'31/32"	170'	1.415	0	5.46	3.10
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"		300.00	1.03	-2'8-1/32"	-1'6-15/32"	153'	1.384	0	0.00	0.00
Panel		300.00	1.69	-3'6-15/32"	-2'15/32"	156'	1.390	0	54.56	55.31
Ericsson RRUS 11		300.00	0.72	-2'8-1/32"	-1'6-15/32"	156'	1.390	0	14.26	10.32
Raycap DC6-48-60-18-8F		0.00	0.09	0'	0'	156'	1.390	0	2.99	2.99
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7"		60.00	1.03	2'8-1/32"	-1'6-15/32"	153'	1.384	0	0.00	0.00
Panel		60.00	1.69	3'6-15/32"	-2'15/32"	156'	1.390	0	54.56	55.31
Ericsson RRUS 11		60.00	0.72	2'8-1/32"	-1'6-15/32"	156'	1.390	0	14.26	10.32

inxTower Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job 150ft of 170ft.90mph							Page 19 of 33	
	Project Brookfield CT							Date 17:04:39 01/11/17	
	Client Ambar							Designed by Chunhui Song	

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _z ksf	C _{4AC} Front ft ²	C _{4AC} Side ft ²	t _z in
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	1.03	0'	3'31/32"	153'	1.384	0	0.00	0.00	1.75
Ericsson RRUS 11	180.00	0.72	0'	3'31/32"	156'	1.390	0	54.56	55.31	1.75
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	1.02	-2'8-17/3 2"	-1'6-27/3 2"	143'	1.365	0	0.00	0.00	1.74
Ericsson RRUS 11	300.00	1.68	-3'6-31/3 2"	-2'27/32"	146'	1.371	0	54.50	55.21	1.74
Ericsson RRUS 11	300.00	1.43	-2'8-17/3 2"	-1'6-27/3 2"	146'	1.371	0	28.49	20.58	1.74
Raycap DC6-48-60-18-8F Ericsson RRUS A2 Module	300.00	0.21	-2'8-17/3 2"	-1'6-27/3 2"	146'	1.371	0	5.44	3.07	1.74
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	1.02	2'8-17/32"	-1'6-27/3 2"	143'	1.365	0	0.00	0.00	1.74
Ericsson RRUS 11	60.00	1.68	3'6-31/32"	-2'27/32"	146'	1.371	0	54.50	55.21	1.74
Ericsson RRUS 11	60.00	1.43	2'8-17/32"	-1'6-27/3 2"	146'	1.371	0	28.49	20.58	1.74
Raycap DC6-48-60-18-8F Ericsson RRUS A2 Module	60.00	0.21	2'8-17/32"	-1'6-27/3 2"	146'	1.371	0	5.44	3.07	1.74
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	1.02	0'	3'1-9/16"	143'	1.365	0	0.00	0.00	1.74
Ericsson RRUS 11	180.00	1.68	0'	4'1-9/16"	146'	1.371	0	54.50	55.21	1.74
Ericsson RRUS 11	180.00	1.43	0'	3'1-9/16"	146'	1.371	0	28.49	20.58	1.74
Raycap DC6-48-60-18-8F Ericsson RRUS A2 Module	60.00	0.21	0'	3'1-9/16"	146'	1.371	0	5.44	3.07	1.74
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	1.02	-2'9-27/3 2"	-1'7-9/16"	133'	1.344	0	0.00	0.00	1.73
Ericsson RRUS 11	300.00	1.67	-3'8-9/32"	-2'1-9/16"	136'	1.350	0	54.44	55.10	1.73
Ericsson RRUS 11	300.00	0.71	-2'9-27/3 2"	-1'7-9/16"	136'	1.350	0	14.22	10.26	1.73
Raycap DC6-48-60-18-8F PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.09	0'	0'	136'	1.350	0	2.98	2.98	1.73
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.04	0'	0'	136'	1.350	0	0.55	0.55	1.73
Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	300.00	0.09	-2'9-27/3 2"	-1'7-9/16"	136'	1.350	0	1.20	0.57	1.73
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	1.02	2'9-27/32"	-1'7-9/16"	133'	1.344	0	0.00	0.00	1.73
Ericsson RRUS 11	60.00	1.67	3'8-9/32"	-2'1-9/16"	136'	1.350	0	54.44	55.10	1.73
Ericsson RRUS 11	60.00	0.71	2'9-27/32"	-1'7-9/16"	136'	1.350	0	14.22	10.26	1.73
Raycap DC6-48-60-18-8F PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.09	0'	0'	136'	1.350	0	2.98	2.98	1.73
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.02	0'	0'	136'	1.350	0	0.28	0.28	1.73
Dplexers 5.8x6.5x1.5 FD9R6004/IC-3L	60.00	0.09	2'9-27/32"	-1'7-9/16"	136'	1.350	0	1.20	0.57	1.73
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	1.02	0'	3'13.1/8"	133'	1.344	0	0.00	0.00	1.73

ttxTower	Job 150ft of 170ft.90mph	Page 20 of 33
<i>Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701</i>	Project Brookfield CT	Date 17:04:39 01/11/17
Client	Ambor	Designed by Chunhui Song

Discrete Appurtenance Pressures - Service $G_H = 1.100$

tnxTower Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job	150ft of 170ft.90mph	Page	21 of 33
	Project	Brookfield CT	Date	17:04:39 01/11/17
	Client	Ambor	Designed by	Chunhui Song

Description	Aiming Azimuth °	Weight	Offset _x	Offset _z	z	K _z	q _x	C _{AC} Front ft ²	C _{AC} Side ft ²
		K	ft	ft	ft	ksf			
Ericsson RRUS A2 Module	300.00	0.06	-2'8-1/32"	-1'6-15/32"	170'	1.415	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'8-1/32"	-1'6-15/32"	166'	1.408	0	11.00	11.00
Ericsson RRUS 11	60.00	0.24	2'8-1/32"	-1'6-15/32"	170'	1.415	0	11.19	6.27
Raycap DC6-48-60-18-8F	60.00	0.02	11-9/32"	-6-15/32"	170'	1.415	0	2.22	2.22
Ericsson RRUS A2 Module	60.00	0.06	2'8-1/32"	-1'6-15/32"	170'	1.415	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0'	3'31/32"	166'	1.408	0	11.00	11.00
Ericsson RRUS 11	180.00	0.28	0'	4'31/32"	170'	1.415	0	45.87	37.91
Raycap DC6-48-60-18-8F	180.00	0.02	0'	1'31/32"	170'	1.415	0	2.22	2.22
Ericsson RRUS A2 Module	180.00	0.06	0'	3'31/32"	170'	1.415	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.50	-2'8-1/32"	-1'6-15/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11	300.00	0.28	-3'6-15/32"	-2'15/32"	156'	1.390	0	45.87	37.91
Ericsson RRUS 11	300.00	0.24	-2'8-1/32"	-1'6-15/32"	156'	1.390	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	156'	1.390	0	2.22	2.22
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'8-1/32"	-1'6-15/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11	60.00	0.28	3'6-15/32"	-2'15/32"	156'	1.390	0	45.87	37.91
Ericsson RRUS 11	60.00	0.24	2'8-1/32"	-1'6-15/32"	156'	1.390	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0'	3'31/32"	153'	1.384	0	11.00	11.00
Ericsson RRUS 11	180.00	0.28	0'	4'31/32"	156'	1.390	0	45.87	37.91
Ericsson RRUS 11	180.00	0.24	0'	3'31/32"	156'	1.390	0	11.19	6.27
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.50	-2'8-17/32"	-1'6-27/32"	143'	1.365	0	11.00	11.00
Ericsson RRUS 11	300.00	0.28	-3'6-31/32"	-2'27/32"	146'	1.371	0	45.87	37.91
Ericsson RRUS 11	300.00	0.48	-2'8-17/32"	-1'6-27/32"	146'	1.371	0	22.37	12.54
Raycap DC6-48-60-18-8F	0.00	0.04	0'	0'	146'	1.371	0	4.44	4.44
Ericsson RRUS A2 Module	300.00	0.06	-2'8-17/32"	-1'6-27/32"	146'	1.371	0	4.13	1.50
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'8-17/32"	-1'6-27/32"	143'	1.365	0	11.00	11.00
Ericsson RRUS 11	60.00	0.28	3'6-31/32"	-2'27/32"	146'	1.371	0	45.87	37.91
Ericsson RRUS 11	60.00	0.48	2'8-17/32"	-1'6-27/32"	146'	1.371	0	22.37	12.54
Raycap DC6-48-60-18-8F	0.00	0.02	0'	0'	146'	1.371	0	2.22	2.22
Ericsson RRUS A2 Module	60.00	0.06	2'8-17/32"	-1'6-27/32"	146'	1.371	0	4.13	1.50
12.6' Low Profile	180.00	0.50	0'	3'1-9/16"	143'	1.365	0	11.00	11.00

tnxTower Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job 150ft of 170ft.90mph							Page 22 of 33
	Project Brookfield CT							Date 17:04:39 01/11/17
	Client Ambar							Designed by Chunhui Song

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _z ksf	C _{AAC} Front ft ²	C _{AAC} Side ft ²
Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.28	0' 4'1-9/16"	146'	1.371	0	45.87	37.91	
Ericsson RRUS 11 Raycap DC6-48-60-18-8F	180.00	0.48	0' 3'1-9/16"	146'	1.371	0	22.37	12.54	
Ericsson RRUS A2 Module	0.00	0.02	0' 0'	146'	1.371	0	2.22	2.22	
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.06	0' 3'1-9/16"	146'	1.371	0	4.13	1.50	
Ericsson RRUS 11 12.6' Low Profile	300.00	0.50	-2'9-27/3 2"	-1'7-9/16"	133'	1.344	0	11.00	11.00
Ericsson RRUS 11 Raycap DC6-48-60-18-8F	300.00	0.28	-3'8-9/32"	-2'1-9/16"	136'	1.350	0	45.87	37.91
PCTEL GPS-TMG-HR-26N 5"x3.2"	300.00	0.24	-2'9-27/3 2"	-1'7-9/16"	136'	1.350	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0' 0'	136'	1.350	0	2.22	2.22	
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.02	0' 0'	136'	1.350	0	0.16	0.16	
Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	300.00	0.06	-2'9-27/3 2"	-1'7-9/16"	136'	1.350	0	0.63	0.17
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'9-27/32" 2"	-1'7-9/16"	133'	1.344	0	11.00	11.00
Ericsson RRUS 11 Ericsson RRUS 11 Raycap DC6-48-60-18-8F	60.00	0.28	3'8-9/32"	-2'1-9/16"	136'	1.350	0	45.87	37.91
PCTEL GPS-TMG-HR-26N 5"x3.2"	60.00	0.24	2'9-27/32" 2"	-1'7-9/16"	136'	1.350	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0' 0'	136'	1.350	0	2.22	2.22	
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.01	0' 0'	136'	1.350	0	0.08	0.08	
Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	60.00	0.06	2'9-27/32" 2"	-1'7-9/16"	136'	1.350	0	0.63	0.17
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0' 3'3-1/8"	133'	1.344	0	11.00	11.00	
Ericsson RRUS 11 Ericsson RRUS 11 Raycap DC6-48-60-18-8F	180.00	0.28	0' 4'3-1/8"	136'	1.350	0	45.87	37.91	
PCTEL GPS-TMG-HR-26N 5"x3.2"	180.00	0.24	0' 3'3-1/8"	136'	1.350	0	11.19	6.27	
Raycap DC6-48-60-18-8F	0.00	0.02	0' 0'	136'	1.350	0	2.22	2.22	
PCTEL GPS-TMG-HR-26N 5"x3.2"	0.00	0.01	0' 0'	136'	1.350	0	0.08	0.08	
Diplexers 5.8x6.5x1.5 FD9R6004/1C-3L	180.00	0.06	0' 3'3-1/8"	136'	1.350	0	0.63	0.17	
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	300.00	0.50	-2'11-1/3 2"	-1'8-5/32"	123'	1.322	0	11.00	11.00
Ericsson RRUS 11 Ericsson RRUS 11 Raycap DC6-48-60-18-8F	300.00	0.28	-3'9-3/8" 2"	-2'2-5/32"	126'	1.329	0	45.87	37.91
PCTEL GPS-TMG-HR-26N 5"x3.2"	300.00	0.24	-2'11-1/3 2"	-1'8-5/32"	126'	1.329	0	11.19	6.27
Raycap DC6-48-60-18-8F	0.00	0.02	0' 0'	126'	1.329	0	2.22	2.22	
E15S09P78 (TMA)	300.00	0.08	-2'11-1/3 2"	-1'8-5/32"	126'	1.329	0	2.99	2.10
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	60.00	0.50	2'11-1/32" 2"	-1'8-5/32"	123'	1.322	0	11.00	11.00
Ericsson RRUS 11 Ericsson RRUS 11 Raycap DC6-48-60-18-8F	60.00	0.28	3'9-3/8" 2"	-2'2-5/32"	126'	1.329	0	45.87	37.91
PCTEL GPS-TMG-HR-26N 5"x3.2"	60.00	0.24	2'11-1/32" 2"	-1'8-5/32"	126'	1.329	0	11.19	6.27

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job 150ft of 170ft.90mph						Page 23 of 33
	Project Brookfield CT						Date 17:04:39 01/11/17
	Client Ambor						Designed by Chunhui Song

Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	z ft	K _z	q _z ksf	C _A A _C Front ft ²	C _A A _C Side ft ²
E15S09P78 (TMA)	60.00	0.08	2'11-1/32"	-1'8-5/32"	126'	1.329	0	2.99	2.10
12.6' Low Profile Platform w/Rails Sector Generic 96"x12"x7" Panel	180.00	0.50	0' 34-7/16"	123'	1.322	0	11.00	11.00	
Ericsson RRUS 11	180.00	0.28	0' 44-7/16"	126'	1.329	0	45.87	37.91	
E15S09P78 (TMA)	180.00	0.24	0' 34-7/16"	126'	1.329	0	11.19	6.27	
21.9' Omni	180.00	0.08	0' 34-7/16"	126'	1.329	0	2.99	2.10	
	180.00	0.01	0' 35-3/4"	75'	1.191	0	3.00	3.00	
Sum Weight:		17.15							

Dish Pressures - No Ice

Elevation ft	Dish Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	K _z	A _A ft ²	q _z ksf
170'	1' HP	0.00	0.02	0'	0'	1.415	0.79	0
60'	2ft Dish	0.00	0.03	0'	0'	1.137	3.14	0
	Sum Weight:	0.05						

Dish Pressures - With Ice

Elevation ft	Dish Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	K _z	A _A ft ²	q _z ksf	t _z in
170'	1' HP	0.00	0.06	0'	0'	1.415	1.27	0	1.77
60'	2ft Dish	0.00	0.09	0'	0'	1.137	3.99	0	1.59
	Sum Weight:	0.14							

Dish Pressures - Service

Elevation ft	Dish Description	Aiming Azimuth °	Weight K	Offset _x ft	Offset _z ft	K _z	A _A ft ²	q _z ksf
170'	1' HP	0.00	0.02	0'	0'	1.415	0.79	0
60'	2ft Dish	0.00	0.03	0'	0'	1.137	3.14	0
	Sum Weight:	0.05						

Force Totals

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M _x kip·ft	Sum of Overturning Moments, M _z kip·ft	Sum of Torques kip·ft
Low Wind 1	24.55					

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Bracing Weight	0.00					
Total Member Self-Weight	34.55			0.03	0.02	
Total Weight	60.63			0.03	0.02	
Wind 0 deg - No Ice		0.00	-34.34	-4295.64	0.02	-0.05
Wind 30 deg - No Ice		17.17	-29.74	-3720.13	-2147.82	0.08
Wind 60 deg - No Ice		29.74	-17.17	-2147.80	-3720.15	0.18
Wind 90 deg - No Ice		34.34	0.00	0.03	-4295.66	0.24
Wind 120 deg - No Ice		29.74	17.17	2147.87	-3720.15	0.23
Wind 150 deg - No Ice		17.17	29.74	3720.20	-2147.82	0.16
Wind 180 deg - No Ice		0.00	34.34	4295.71	0.02	0.05
Wind 210 deg - No Ice		-17.17	29.74	3720.20	2147.86	-0.08
Wind 240 deg - No Ice		-29.74	17.17	2147.87	3720.18	-0.18
Wind 270 deg - No Ice		-34.34	0.00	0.03	4295.70	-0.24
Wind 300 deg - No Ice		-29.74	-17.17	-2147.80	3720.18	-0.23
Wind 330 deg - No Ice		-17.17	-29.74	-3720.13	2147.86	-0.16
Member Ice	15.32					
Total Weight Ice	191.61			0.08	0.08	
Wind 0 deg - Ice		0.00	-14.59	-1703.09	0.08	-0.02
Wind 30 deg - Ice		7.29	-12.63	-1474.91	-851.50	-0.02
Wind 60 deg - Ice		12.63	-7.29	-851.51	-1474.90	-0.02
Wind 90 deg - Ice		14.59	0.00	0.08	-1703.08	-0.01
Wind 120 deg - Ice		12.63	7.29	851.66	-1474.90	0.00
Wind 150 deg - Ice		7.29	12.63	1475.06	-851.50	0.01
Wind 180 deg - Ice		0.00	14.59	1703.24	0.08	0.02
Wind 210 deg - Ice		-7.29	12.63	1475.06	851.67	0.02
Wind 240 deg - Ice		-12.63	7.29	851.66	1475.07	0.02
Wind 270 deg - Ice		-14.59	0.00	0.08	1703.25	0.01
Wind 300 deg - Ice		-12.63	-7.29	-851.51	1475.07	0.00
Wind 330 deg - Ice		-7.29	-12.63	-1474.91	851.67	-0.01
Total Weight	60.63			0.03	0.02	
Wind 0 deg - Service		0.00	-13.66	-1708.19	0.02	-0.02
Wind 30 deg - Service		6.83	-11.83	-1479.33	-854.09	0.03
Wind 60 deg - Service		11.83	-6.83	-854.08	-1479.35	0.07
Wind 90 deg - Service		13.66	0.00	0.03	-1708.20	0.10
Wind 120 deg - Service		11.83	6.83	854.15	-1479.35	0.09
Wind 150 deg - Service		6.83	11.83	1479.40	-854.09	0.07
Wind 180 deg - Service		0.00	13.66	1708.26	0.02	0.02
Wind 210 deg - Service		-6.83	11.83	1479.40	854.13	-0.03
Wind 240 deg - Service		-11.83	6.83	854.15	1479.38	-0.07
Wind 270 deg - Service		-13.66	0.00	0.03	1708.24	-0.10
Wind 300 deg - Service		-11.83	-6.83	-854.08	1479.38	-0.09
Wind 330 deg - Service		-6.83	-11.83	-1479.33	854.13	-0.07

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice

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Comb. No.	Description
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	170 - 150	Pole	Max Tension	36	0.00	-0.00	-0.00
			Max. Compression	26	-28.71	0.09	0.05
			Max. Mx	20	-7.14	227.20	0.01
			Max. My	2	-7.14	0.01	227.19
			Max. Vy	20	-17.94	227.20	0.01
			Max. Vx	2	-17.94	0.01	227.19
			Max. Torque	16		-0.10	
L2	150 - 126.279	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-64.87	0.09	0.05
			Max. Mx	20	-16.89	748.79	0.01
			Max. My	2	-16.89	0.02	748.78
			Max. Vy	20	-36.13	748.79	0.01

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L3	126.279 - 94.9479	Pole	Max. Vx	14	36.13	0.02	-748.76
			Max. Torque	16			-0.10
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-102.23	0.09	0.05
			Max. Mx	20	-28.24	2066.65	-0.00
			Max. My	2	-28.24	0.02	2066.64
			Max. Vy	20	-46.18	2066.65	-0.00
			Max. Vx	14	46.18	0.02	-2066.61
			Max. Torque	16			-0.10
			Max Tension	1	0.00	0.00	0.00
L4	94.9479 - 63.7161	Pole	Max. Compression	26	-132.26	0.09	-0.08
			Max. Mx	20	-39.42	3509.81	-0.04
			Max. My	14	-39.42	0.02	-3509.83
			Max. Vy	20	-49.14	3509.81	-0.04
			Max. Vx	14	49.14	0.02	-3509.83
			Max. Torque	9			-0.38
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-165.09	0.09	-0.08
			Max. Mx	20	-52.90	5070.91	-0.04
			Max. My	14	-52.90	0.02	-5070.93
L5	63.7161 - 31.7943	Pole	Max. Vy	20	-52.06	5070.91	-0.04
			Max. Vx	14	52.06	0.02	-5070.93
			Max. Torque	9			-0.38
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-206.32	0.09	-0.08
			Max. Mx	20	-72.73	7147.97	-0.04
			Max. My	14	-72.73	0.02	-7147.99
			Max. Vy	20	-54.99	7147.97	-0.04
			Max. Vx	14	54.99	0.02	-7147.99
			Max. Torque	9			-0.38
L6	31.7943 - 1	Pole	Max. Compression	26	-206.32	0.09	-0.08
			Max. Mx	20	-72.73	7147.97	-0.04
			Max. My	14	-72.73	0.02	-7147.99
			Max. Vy	20	-54.99	7147.97	-0.04
			Max. Vx	14	54.99	0.02	-7147.99
			Max. Torque	9			-0.38
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-206.32	0.09	-0.08
			Max. Mx	20	-72.73	7147.97	-0.04
			Max. My	14	-72.73	0.02	-7147.99

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	36	206.32	14.59	0.00
	Max. H _x	20	72.76	54.95	0.00
	Max. H _z	2	72.76	0.00	54.95
	Max. M _x	2	7147.90	0.00	54.95
	Max. M _z	8	7147.92	-54.95	0.00
	Max. Torsion	21	0.38	54.95	0.00
	Min. Vert	23	54.57	47.59	27.47
	Min. H _x	8	72.76	-54.95	0.00
	Min. H _z	14	72.76	0.00	-54.95
	Min. M _x	14	-7147.99	0.00	-54.95
Min. M _z	20	-7147.97	54.95	0.00	
	Min. Torsion	9	-0.38	-54.95	0.00

Tower Mast Reaction Summary

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Load Combination	Vertical	Shear _x	Shear _z	Overturning Moment, M _x	Overturning Moment, M _z	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	60.63	0.00	0.00	0.03	0.02	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice	72.76	0.00	-54.95	-7147.90	0.02	-0.08
0.9 Dead+1.6 Wind 0 deg - No Ice	54.57	0.00	-54.95	-7072.83	0.02	-0.08
1.2 Dead+1.6 Wind 30 deg - No Ice	72.76	27.47	-47.59	-6190.26	-3573.95	0.12
0.9 Dead+1.6 Wind 30 deg - No Ice	54.57	27.47	-47.59	-6125.25	-3536.41	0.12
1.2 Dead+1.6 Wind 60 deg - No Ice	72.76	47.59	-27.47	-3573.93	-6190.28	0.29
0.9 Dead+1.6 Wind 60 deg - No Ice	54.57	47.59	-27.47	-3536.40	-6125.26	0.29
1.2 Dead+1.6 Wind 90 deg - No Ice	72.76	54.95	0.00	0.04	-7147.92	0.38
0.9 Dead+1.6 Wind 90 deg - No Ice	54.57	54.95	0.00	0.03	-7072.84	0.38
1.2 Dead+1.6 Wind 120 deg - No Ice	72.76	47.59	27.47	3574.01	-6190.28	0.37
0.9 Dead+1.6 Wind 120 deg - No Ice	54.57	47.59	27.47	3536.46	-6125.26	0.37
1.2 Dead+1.6 Wind 150 deg - No Ice	72.76	27.47	47.59	6190.35	-3573.95	0.26
0.9 Dead+1.6 Wind 150 deg - No Ice	54.57	27.47	47.59	6125.31	-3536.41	0.26
1.2 Dead+1.6 Wind 180 deg - No Ice	72.76	0.00	54.95	7147.99	0.02	0.08
0.9 Dead+1.6 Wind 180 deg - No Ice	54.57	0.00	54.95	7072.89	0.02	0.08
1.2 Dead+1.6 Wind 210 deg - No Ice	72.76	-27.47	47.59	6190.35	3574.00	-0.12
0.9 Dead+1.6 Wind 210 deg - No Ice	54.57	-27.47	47.59	6125.31	3536.45	-0.12
1.2 Dead+1.6 Wind 240 deg - No Ice	72.76	-47.59	27.47	3574.02	6190.33	-0.29
0.9 Dead+1.6 Wind 240 deg - No Ice	54.57	-47.59	27.47	3536.46	6125.30	-0.29
1.2 Dead+1.6 Wind 270 deg - No Ice	72.76	-54.95	0.00	0.04	7147.97	-0.38
0.9 Dead+1.6 Wind 270 deg - No Ice	54.57	-54.95	0.00	0.03	7072.88	-0.38
1.2 Dead+1.6 Wind 300 deg - No Ice	72.76	-47.59	-27.47	-3573.93	6190.33	-0.37
0.9 Dead+1.6 Wind 300 deg - No Ice	54.57	-47.59	-27.47	-3536.40	6125.30	-0.37
1.2 Dead+1.6 Wind 330 deg - No Ice	72.76	-27.47	-47.59	-6190.26	3574.00	-0.26
0.9 Dead+1.6 Wind 330 deg - No Ice	54.57	-27.47	-47.59	-6125.25	3536.45	-0.26
1.2 Dead+1.0 Ice+1.0 Temp	206.32	0.00	0.00	0.08	0.09	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	206.32	0.00	-14.59	-1950.17	0.12	-0.02
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	206.32	7.29	-12.63	-1688.89	-975.01	-0.03
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	206.32	12.63	-7.29	-975.04	-1688.85	-0.02
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	206.32	14.59	0.00	0.09	-1950.14	-0.01
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	206.32	12.63	7.29	975.21	-1688.85	-0.00

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job	150ft of 170ft.90mph	Page	28 of 33
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	Client	Ambor	Designed by	Chunhui Song

Load Combination	Vertical	Shear _x	Shear _z	Overshadowing Moment, M _x	Overshadowing Moment, M _z	Torque
	K	K	K	kip·ft	kip·ft	kip·ft
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	206.32	7.29	12.63	1689.06	-975.01	0.01
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	206.32	0.00	14.59	1950.34	0.12	0.02
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	206.32	-7.29	12.63	1689.06	975.25	0.03
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	206.32	-12.63	7.29	975.21	1689.09	0.02
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	206.32	-14.59	0.00	0.09	1950.37	0.01
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	206.32	-12.63	-7.29	-975.04	1689.09	0.00
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	206.32	-7.29	-12.63	-1688.89	975.25	-0.01
Dead+Wind 0 deg - Service	60.63	0.00	-13.66	-1767.80	0.02	-0.02
Dead+Wind 30 deg - Service	60.63	6.83	-11.83	-1530.96	-883.90	0.03
Dead+Wind 60 deg - Service	60.63	11.83	-6.83	-883.88	-1530.97	0.07
Dead+Wind 90 deg - Service	60.63	13.66	0.00	0.04	-1767.82	0.10
Dead+Wind 120 deg - Service	60.63	11.83	6.83	883.96	-1530.97	0.09
Dead+Wind 150 deg - Service	60.63	6.83	11.83	1531.03	-883.90	0.07
Dead+Wind 180 deg - Service	60.63	0.00	13.66	1767.87	0.02	0.02
Dead+Wind 210 deg - Service	60.63	-6.83	11.83	1531.03	883.94	-0.03
Dead+Wind 240 deg - Service	60.63	-11.83	6.83	883.96	1531.01	-0.07
Dead+Wind 270 deg - Service	60.63	-13.66	0.00	0.04	1767.86	-0.10
Dead+Wind 300 deg - Service	60.63	-11.83	-6.83	-883.88	1531.01	-0.09
Dead+Wind 330 deg - Service	60.63	-6.83	-11.83	-1530.96	883.94	-0.07

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-60.63	0.00	0.00	60.63	0.00	0.000%
2	0.00	-72.76	-54.95	0.00	72.76	54.95	0.000%
3	0.00	-54.57	-54.95	0.00	54.57	54.95	0.000%
4	27.47	-72.76	-47.59	-27.47	72.76	47.59	0.000%
5	27.47	-54.57	-47.59	-27.47	54.57	47.59	0.000%
6	47.59	-72.76	-27.47	-47.59	72.76	27.47	0.000%
7	47.59	-54.57	-27.47	-47.59	54.57	27.47	0.000%
8	54.95	-72.76	0.00	-54.95	72.76	0.00	0.000%
9	54.95	-54.57	0.00	-54.95	54.57	0.00	0.000%
10	47.59	-72.76	27.47	-47.59	72.76	-27.47	0.000%
11	47.59	-54.57	27.47	-47.59	54.57	-27.47	0.000%
12	27.47	-72.76	47.59	-27.47	72.76	-47.59	0.000%
13	27.47	-54.57	47.59	-27.47	54.57	-47.59	0.000%
14	0.00	-72.76	54.95	0.00	72.76	-54.95	0.000%
15	0.00	-54.57	54.95	0.00	54.57	-54.95	0.000%
16	-27.47	-72.76	47.59	27.47	72.76	-47.59	0.000%
17	-27.47	-54.57	47.59	27.47	54.57	-47.59	0.000%
18	-47.59	-72.76	27.47	47.59	72.76	-27.47	0.000%
19	-47.59	-54.57	27.47	47.59	54.57	-27.47	0.000%
20	-54.95	-72.76	0.00	54.95	72.76	0.00	0.000%
21	-54.95	-54.57	0.00	54.95	54.57	0.00	0.000%
22	-47.59	-72.76	-27.47	47.59	72.76	27.47	0.000%
23	-47.59	-54.57	-27.47	47.59	54.57	27.47	0.000%
24	-27.47	-72.76	-47.59	27.47	72.76	47.59	0.000%
25	-27.47	-54.57	-47.59	27.47	47.59	47.59	0.000%
26	0.00	-206.32	0.00	0.00	206.32	0.00	0.000%

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	Project	Brookfield CT	Date	17:04:39 01/11/17
	Client	Ambor	Designed by	Chunhui Song

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
27	0.00	-206.32	-14.59	0.00	206.32	14.59	0.000%
28	7.29	-206.32	-12.63	-7.29	206.32	12.63	0.000%
29	12.63	-206.32	-7.29	-12.63	206.32	7.29	0.000%
30	14.59	-206.32	0.00	-14.59	206.32	0.00	0.000%
31	12.63	-206.32	7.29	-12.63	206.32	-7.29	0.000%
32	7.29	-206.32	12.63	-7.29	206.32	-12.63	0.000%
33	0.00	-206.32	14.59	0.00	206.32	-14.59	0.000%
34	-7.29	-206.32	12.63	7.29	206.32	-12.63	0.000%
35	-12.63	-206.32	7.29	12.63	206.32	-7.29	0.000%
36	-14.59	-206.32	0.00	14.59	206.32	0.00	0.000%
37	-12.63	-206.32	-7.29	12.63	206.32	7.29	0.000%
38	-7.29	-206.32	-12.63	7.29	206.32	12.63	0.000%
39	0.00	-60.63	-13.66	0.00	60.63	13.66	0.000%
40	6.83	-60.63	-11.83	-6.83	60.63	11.83	0.000%
41	11.83	-60.63	-6.83	-11.83	60.63	6.83	0.000%
42	13.66	-60.63	0.00	-13.66	60.63	0.00	0.000%
43	11.83	-60.63	6.83	-11.83	60.63	-6.83	0.000%
44	6.83	-60.63	11.83	-6.83	60.63	-11.83	0.000%
45	0.00	-60.63	13.66	0.00	60.63	-13.66	0.000%
46	-6.83	-60.63	11.83	6.83	60.63	-11.83	0.000%
47	-11.83	-60.63	6.83	11.83	60.63	-6.83	0.000%
48	-13.66	-60.63	0.00	13.66	60.63	0.00	0.000%
49	-11.83	-60.63	-6.83	11.83	60.63	6.83	0.000%
50	-6.83	-60.63	-11.83	6.83	60.63	11.83	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00002914
3	Yes	5	0.00000001	0.00000950
4	Yes	7	0.00000001	0.00000772
5	Yes	6	0.00000001	0.00004791
6	Yes	7	0.00000001	0.00000773
7	Yes	6	0.00000001	0.00004795
8	Yes	5	0.00000001	0.00002902
9	Yes	5	0.00000001	0.00000946
10	Yes	7	0.00000001	0.00000774
11	Yes	6	0.00000001	0.00004800
12	Yes	7	0.00000001	0.00000772
13	Yes	6	0.00000001	0.00004787
14	Yes	5	0.00000001	0.00002914
15	Yes	5	0.00000001	0.00000950
16	Yes	7	0.00000001	0.00000773
17	Yes	6	0.00000001	0.00004798
18	Yes	7	0.00000001	0.00000773
19	Yes	6	0.00000001	0.00004794
20	Yes	5	0.00000001	0.00002902
21	Yes	5	0.00000001	0.00000946
22	Yes	7	0.00000001	0.00000772
23	Yes	6	0.00000001	0.00004788
24	Yes	7	0.00000001	0.00000774
25	Yes	6	0.00000001	0.00004801
26	Yes	4	0.00000001	0.00000001
27	Yes	7	0.00000001	0.00006452

tnxTower Bennett & Pless 550 River Drive North Sioux City, SD 57049 Phone: 605-540-4621 FAX: 678-990-8701	Job	150ft of 170ft.90mph	Page	30 of 33
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28	Yes	7	0.00000001	0.00009787
29	Yes	7	0.00000001	0.00009798
30	Yes	7	0.00000001	0.00006451
31	Yes	7	0.00000001	0.00009792
32	Yes	7	0.00000001	0.00009789
33	Yes	7	0.00000001	0.00006452
34	Yes	7	0.00000001	0.00009802
35	Yes	7	0.00000001	0.00009792
36	Yes	7	0.00000001	0.00006453
37	Yes	7	0.00000001	0.00009797
38	Yes	7	0.00000001	0.00009800
39	Yes	5	0.00000001	0.00000815
40	Yes	6	0.00000001	0.00000451
41	Yes	6	0.00000001	0.00000452
42	Yes	5	0.00000001	0.00000814
43	Yes	6	0.00000001	0.00000452
44	Yes	6	0.00000001	0.00000450
45	Yes	5	0.00000001	0.00000815
46	Yes	6	0.00000001	0.00000452
47	Yes	6	0.00000001	0.00000451
48	Yes	5	0.00000001	0.00000814
49	Yes	6	0.00000001	0.00000451
50	Yes	6	0.00000001	0.00000453

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	170 - 150	34.45	48	1.94	0.00
L2	150 - 126.279	26.43	48	1.85	0.00
L3	131.036 - 94.9479	19.57	48	1.58	0.00
L4	100.789 - 63.7161	10.90	45	1.11	0.00
LS	70.5078 - 31.7943	5.05	45	0.71	0.00
L6	39.7188 - 1	1.52	46	0.35	0.00

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
170'	1' HP	48	34.45	1.94	0.00	35827
166'	12.6' Low Profile Platform w/Rails Sector	48	32.81	1.93	0.00	26870
156'	12.6' Low Profile Platform w/Rails Sector	48	28.78	1.89	0.00	7677
146'	12.6' Low Profile Platform w/Rails Sector	48	24.90	1.80	0.00	4943
136'	12.6' Low Profile Platform w/Rails Sector	48	21.27	1.66	0.00	4119
126'	12.6' Low Profile Platform w/Rails Sector	48	17.92	1.50	0.00	3788
75'	21.9' Omni	45	5.76	0.77	0.00	4785
60'	2ft Dish	45	3.57	0.58	0.00	4805

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job 150ft of 170ft.90mph	Page 31 of 33
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Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	170 - 150	139.13	20	7.85	0.00
L2	150 - 126.279	106.80	20	7.47	0.00
L3	131.036 - 94.9479	79.11	20	6.40	0.00
L4	100.789 - 63.7161	44.09	14	4.49	0.00
L5	70.5078 - 31.7943	20.41	14	2.88	0.00
L6	39.7188 - 1	6.15	14	1.44	0.00

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appartenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
170'	1' HP	20	139.13	7.85	0.00	9192
166'	12.6' Low Profile Platform w/Rails Sector	20	132.55	7.81	0.00	6893
156'	12.6' Low Profile Platform w/Rails Sector	20	116.29	7.65	0.00	1967
146'	12.6' Low Profile Platform w/Rails Sector	20	100.65	7.30	0.00	1262
136'	12.6' Low Profile Platform w/Rails Sector	20	85.97	6.73	0.00	1046
126'	12.6' Low Profile Platform w/Rails Sector	20	72.46	6.07	0.00	957
75'	21.9' Omni	14	23.29	3.11	0.00	1189
60'	2ft Dish	14	14.45	2.36	0.00	1191

Base Plate Design Data

Plate Thickness	Number of Anchor Bolts	Anchor Bolt Size	Actual Allowable Ratio Bolt Tension K	Actual Allowable Ratio Concrete Stress ksi	Actual Allowable Ratio Plate Stress ksi	Actual Allowable Ratio Stiffener Stress ksi	Controlling Condition	Critical Ratio
3.15	20	2.25	171.83 223.65 0.77	3.37 6.12 0.55	32.26 45.00 0.72		Bolt T ✓	0.77

Compression Checks

tnxTower <i>Bennett & Pless</i> <i>550 River Drive</i> <i>North Sioux City, SD 57049</i> <i>Phone: 605-540-4621</i> <i>FAX: 678-990-8701</i>	Job	150ft of 170ft.90mph	Page	32 of 33
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Pole Design Data

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	ϕP _n	Ratio P _u /ϕP _n
	ft		ft	ft		in ²	K	K	ϕP _n
L1	170 - 150 (1)	TP25.98x25.98x0.2	20'	169'	221.5	16.11	-28.56	74.17	0.385
L2	150 - 126.279 (2)	TP33.18x25.98x0.24	23'8-5/8"	169'	181.4	23.62	-16.89	162.20	0.104
L3	126.279 - 94.9479 (3)	TP42.21x31.26x0.31	36'1-3/3 2"	169'	142.4	40.11	-28.24	447.07	0.063
L4	94.9479 - 63.7161 (4)	TP51.06x39.81x0.39	37'27/32 "	169'	117.5	60.73	-39.42	993.07	0.040
L5	63.7161 - 31.7943 (5)	TP59.95x48.21x0.39	38'8-17/ 32"	169'	100.0	71.42	-52.90	1614.82	0.033
L6	31.7943 - 1 (6)	TP68.5x56.76x0.43	38'8-5/8"	169'	83.9	93.57	-72.73	2874.73	0.025

Pole Bending Design Data

Section No.	Elevation	Size	M _{ex}	ϕM _{ex}	Ratio M _{ex} /ϕM _{ex}	M _{ey}	ϕM _{ey}	Ratio M _{ey} /ϕM _{ey}
	ft		kip·ft	kip·ft	ϕM _{ex}	kip·ft	kip·ft	ϕM _{ey}
L1	170 - 150 (1)	TP25.98x25.98x0.2	61.07	585.65	0.104	0.00	585.65	0.000
L2	150 - 126.279 (2)	TP33.18x25.98x0.24	748.79	1041.97	0.719	0.00	1041.97	0.000
L3	126.279 - 94.9479 (3)	TP42.21x31.26x0.31	2066.66	2290.43	0.902	0.00	2290.43	0.000
L4	94.9479 - 63.7161 (4)	TP51.06x39.81x0.39	3509.84	4243.98	0.827	0.00	4243.98	0.000
L5	63.7161 - 31.7943 (5)	TP59.95x48.21x0.39	5070.94	5535.76	0.916	0.00	5535.76	0.000
L6	31.7943 - 1 (6)	TP68.5x56.76x0.43	7148.00	8349.25	0.856	0.00	8349.25	0.000

Pole Shear Design Data

Section No.	Elevation	Size	Actual V _u /K	ϕV _n /K	Ratio V _u /ϕV _n	Actual T _u /kip·ft	ϕT _n /kip·ft	Ratio T _u /ϕT _n
	ft		K	K	ϕV _n	kip·ft	kip·ft	ϕT _n
L1	170 - 150 (1)	TP25.98x25.98x0.2	4.84	552.09	0.009	0.00	1172.72	0.000
L2	150 - 126.279 (2)	TP33.18x25.98x0.24	36.13	804.07	0.045	0.00	2086.50	0.000
L3	126.279 - 94.9479 (3)	TP42.21x31.26x0.31	46.18	1387.94	0.033	0.00	4586.46	0.000
L4	94.9479 - 63.7161 (4)	TP51.06x39.81x0.39	49.14	2123.77	0.023	0.12	8498.33	0.000
L5	63.7161 - 31.7943 (5)	TP59.95x48.21x0.39	52.06	2352.92	0.022	0.12	11085.08	0.000
L6	31.7943 - 1 (6)	TP68.5x56.76x0.43	54.99	2977.96	0.018	0.12	16719.00	0.000

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Pole Interaction Design Data

Section No.	Elevation ft	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{sy}}{\phi M_{nsy}}$	Ratio $\frac{V_u}{\phi V_n}$	Ratio $\frac{T_x}{\phi T_n}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	170 - 150 (1)	0.385	0.104	0.000	0.009	0.000	0.489	1.000	4.8.2 ✓
L2	150 - 126.279 (2)	0.104	0.719	0.000	0.045	0.000	0.825	1.000	4.8.2 ✓
L3	126.279 - 94.9479 (3)	0.063	0.902	0.000	0.033	0.000	0.967	1.000	4.8.2 ✓
L4	94.9479 - 63.7161 (4)	0.040	0.827	0.000	0.023	0.000	0.867	1.000	4.8.2 ✓
L5	63.7161 - 31.7943 (5)	0.033	0.916	0.000	0.022	0.000	0.949	1.000	4.8.2 ✓
L6	31.7943 - 1 (6)	0.025	0.856	0.000	0.018	0.000	0.882	1.000	4.8.2 ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	170 - 150	Pole	TP25.98x25.98x0.2	1	-28.56	74.17	48.9	Pass
L2	150 - 126.279	Pole	TP33.18x25.98x0.24	2	-16.89	162.20	82.5	Pass
L3	126.279 - 94.9479	Pole	TP42.21x31.26x0.31	3	-28.24	447.07	96.7	Pass
L4	94.9479 - 63.7161	Pole	TP51.06x39.81x0.39	4	-39.42	993.07	86.7	Pass
L5	63.7161 - 31.7943	Pole	TP59.95x48.21x0.39	5	-52.90	1614.82	94.9	Pass
L6	31.7943 - 1	Pole	TP68.5x56.76x0.43	6	-72.73	2874.73	88.2	Summary
					Pole (L3)	96.7	Pass	
					Base Plate	76.8	Pass	
					RATING =	96.7	Pass	

Base/Flange Plate	Plate Type	Baseplate
	Pole Diameter	68.5 in
	Pole Thickness	0.43 in
	Plate Diameter	82.28 in
	Plate Thickness	3.15 in
	Plate Fy	50 ksi
	Weld Length	0.3125 in
	ϕ_s Resistance	1201.11 k-in
Applied		528.22 k-in
	#	0

Code Rev. **G** Date **1/11/2017**
 Engineer **CS** Site # **CT777 Brookfield**
 Moment **7148.0 k-ft** Carrier
 Axial **73.0 k**

Bolts	#	20
	Bolt Circle (R)adial / (S)square	76.38 in
	Diameter	2.25 in
	Hole Diameter	2.625 in
	Type	A615-75
	Fy	75 ksi
	Fu	100 ksi
	ϕ_s Resistance	259.82 k
Applied		228.17 k
	#	0

Reinforcement ●	#	0
	#	0
Extra Bolts O		

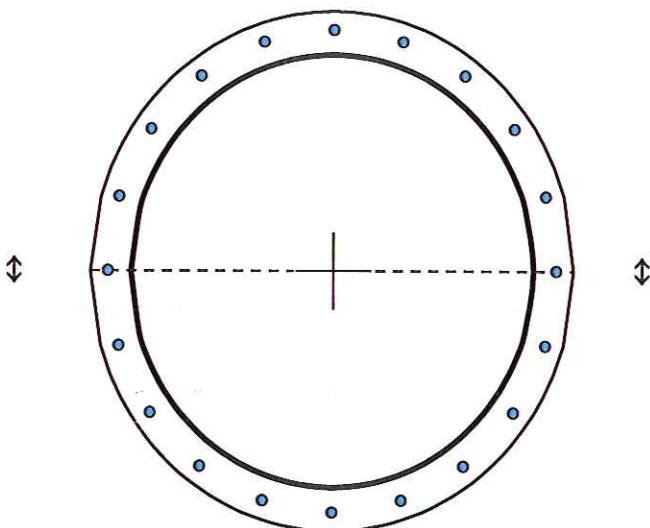


Plate Stress Ratio:

0.44 (Pass)

Bolt Stress Ratio:

0.88 (Pass)

PROJECT No: 16700.021
 PROJECT NAME: CT777 Brookfield
 Ambor/Insite
 DATE: January 11, 2017

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TIA-222-G

SINGLE GLOBAL FOUNDATION WITH PIER(s) CHECKS

Global Tower Reactions		Factored Loads	Calculated Reactions	Factored Resistance		
(@TIA-G) Maximum Moment		7,148.00 k-ft	Disturbing Moment	7,505.5	8,719.5 k-ft	pass 86.1% [GOVERNS]
(@EIA-F) Axial Load		73.00 kips	Maximum Bearing	3.24	9.00 kips	pass 36.0%
Shear Load		55.00 kips	Punching Shear	711.5	1,224.0 kips	pass 58.1%
Pier Rebar Required	(minimum only, use PCACOL for total quantity)			(32) #10 @ 8.84 in **MINIMUM**		
Rebar Required	(checked rebar for 6" min to 24" max spacing)			(42) #10 @ 8.63 in		
				SF=2.32		

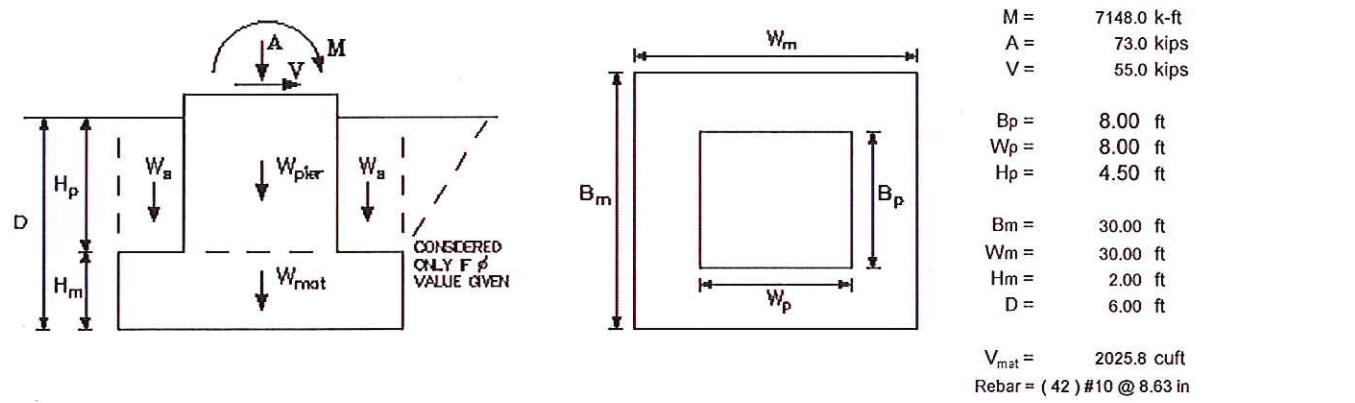
Soil Parameters		Soils Report	Pier Geometry		Pad Geometry	
ϕ		30.0 °	Qty of Piers	1	Width (Bm)	30.00 ft
Water Level		19.00 ft (5.79 m)	Width (Bp)	8.00 ft	Width (Wm)	30.00 ft
Soil Dry Density (γ_{dry})		0.100 kcf (15.7 kN/m³)	Width (Wp)	8.00 ft	Height (Hm)	2.00 ft
Soil Sub Density (γ_{sub})		0.050 kcf (7.85 kN/m³)	Height (Hp)	4.50 ft	Depth (D)	6.00 ft
All. Bearing Pressure		6.000 ksf (287.3 kPa)	Pier Type	R (Rnd or Sq)		
Bearing Safety Factor		2	Conc γ_{dry}	0.150 kcf (23.6)		

Concrete (75.0cuyd)			Calculations		Factored	Allowable
1 Pier	Mat	Soil	Axial Download	73.0	-- kips	
0.50	--	-- ft	Weight of Concrete (not factored)	303.9	-- kips(75.0yds)	
4.00	2.00	4.00 ft	Weight of Soil (not factored)	398.2	-- kips	
0.00	0.00	0.00 ft	Total Download (P)	775.1	-- kips	
25.09	--	-- ft³	Resisting Moment Arm	15.0	-- ft	
200.70	1,800.00	3982.00 ft³	Moment Resistance	8719.5	-- k-ft	
0.00	0	0.00 ft³	(x 0.75, cl 9.4.1)			
Total	226	1800	3982 ft³			

Concrete Reinforcing Design			Bearing Capacity Check	
f_c	3.000 ksi	(20.7 MPa)	Contact Area	900.00 -- ft²
f_y	60.00 ksi	(413.7 MPa)	Calculate eccentricity e	9.68 -- ft [$>L/6$]
Steel (Metric/ASTM)	ASTM	MAT	Calculate ($c = L/2 - e$)	5.32 -- ft
Bar size	10 #	ASTM	1) $q_{max} = P/A \cdot (1+6e/L)$	-- --
	1.270 in²	PIER	2) $q_{max} = 2P/b \cdot c$	3.24 -- ksf [GOV]

Slab Reinforcing		Check for 2-Way Shear (Punching)	
½ Disturbing Moment	3752.75 kip-ft	Shear Area ($b_0 \times d$)	51.73 -- ft²
Ku	283.65	Factored Bearing Stress	0.861 -- ksf
ρ	0.00562	Factored Shear Force	711.46 -- kips
$4/3 \cdot \rho$ if $\rho < \rho_{min}$	0.00749	Factored Shear Resistance	1224.0 -- kips
$\rho_{min} \geq 0.0018$	0.00180	Check for 2-way Shear	0.58 --
As	40.44 in²	(ACI-318)	
Number of bars	42 bars on 8.63 in c/c		

Note: The 1/2 moment is derived from a bending moment diagram that considered the uplift and download components at the exact face width of the tower.



65° Single Band Panel Antenna, 6'

	Antenna
Single Band (MHz)	698–894
Dual Polarization	X
HPBW	65°
Adj. Electrical Downtilt <small>Manual or optional remote control</small>	0°–10°

General specifications:

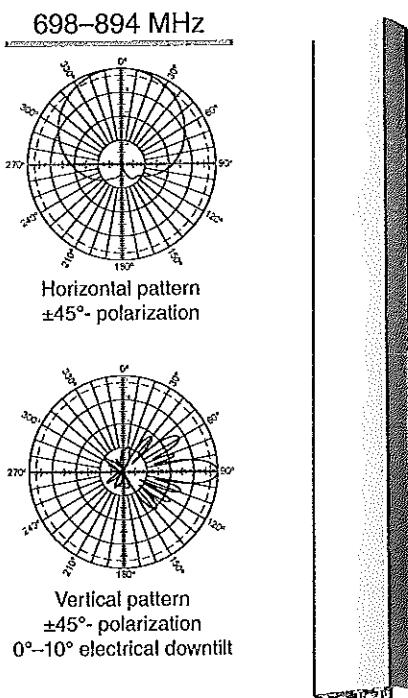
Frequency range	698–894 MHz
VSWR	<1.5:1
Impedance	50 ohms
Intermodulation (2x20w)	IM3: <-150 dBc
Polarization	+45° and -45°
Maximum input power	500 watts per input (at 50°C)
Connector	2 x 7-16 DIN female (long neck) (bottom mounted)
Isolation	>30 dB
Electrical downtilt	0–10 degrees (continuously adjustable)

See reverse for order information.

Specifications:	698–806 MHz	824–894 MHz
Gain	15.5 dBi	16 dBi
Front-to-back ratio	>30 dB (co-polar) 35 dB (average)	>30 dB (co-polar) 35 dB (average)
+45° and -45° polarization horizontal beamwidth	67° (half-power)	65° (half-power)
+45° and -45° polarization vertical beamwidth	11.3° (half-power)	10° (half-power)
Min. sidelobe suppression for first sidelobe above main beam average	0° 5° 10° T 16 17 17 dB 16 19 20 dB	0° 5° 10° T 18 17 16 dB 20 20 20 dB
Cross polar ratio		
Main direction	0° 25 dB (typical)	25 dB (typical)
Sector	±60° >11 dB, Average: 15 dB	>11 dB, Average: 15 dB

IRT specifications:

Logical interface ex factory ¹	3GPP/AISG 2.0
Protocols	AISG 1.1 and 3GPP/AISG 2.0 compliant
Hardware interface ²	2 x 8 pin connector acc. IEC 60130-9; according to AISG: – IRT in (male): Control / Daisy chain in – IRT in (female): Daisy chain out
Power supply	10–30 V
Power consumption	<1 watt (standby) <8.5 watts (motor activated)
Adjustment time (full range)	40 sec.
Adjustment cycles	>50,000
Certification	FCC 15.107 Class B Computing Devices



¹⁾ The protocol of the logical interface can be switched from 3GPP/AISG 2.0 to AISG 1.1 and vice versa with a vendor specific command. Start-up operation of the RCU 86010149 is possible in an RET system supporting AISG 1.1 or supporting 3GPP/AISG 2.0 after performing a layer 2 reset before address assignment. The protocol can also be changed as follows: AISG 1.1 to 3GPP: Enter "3GPP" into the additional data field "Installer's ID" and perform a layer 7 reset or a power reset. 3GPP to AISG 1.1: Enter "AISG 1" into the additional datafield "Installer's ID" and perform a layer 2 reset or a power reset. After switching the protocol any other information can be entered into the "Installer's ID" field.

²⁾ The tightening torque for fixing the connector must be 0.5 – 1.0 Nm ('hand-tightened'). The connector should be tightened by hand only!

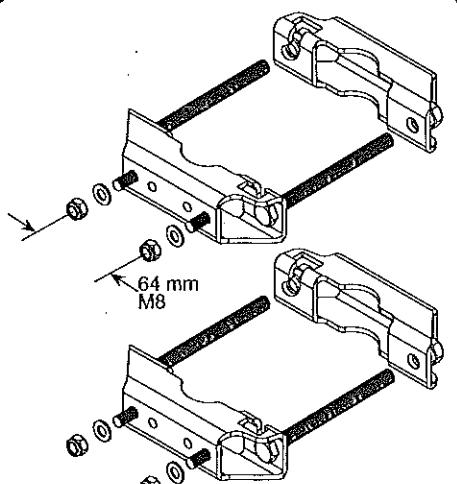


800 10735V01

65° Single Band Panel Antenna, 6'

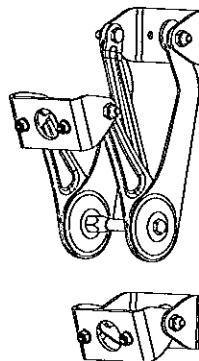
Mechanical specifications:

Weight	30.9 lb (14 kg)	35.3 lb (16 kg) clamps included
Dimensions H x W x D	76.1 x 11.9 x 3.9 inches (1934 x 303 x 99 mm)	
Wind load Front/Side/Rear	at 93 mph (150kph) 203 lbf / 70 lb / 232 lbf (900 N / 310 N / 1030 N)	
Mounting category	H (Heavy)	
Wind survival rating*	150 mph (240 kph)	
Shipping dimensions	81.1 x 12.4 x 4.5 inches (2060 x 315 x 115 mm)	
Shipping weight	39.7 lb (18 kg)	
Mounting bracket	2-point hot-dip galvanized with stainless steel hardware for 2 to 4.5 inch (50 to 115 mm) OD masts.	



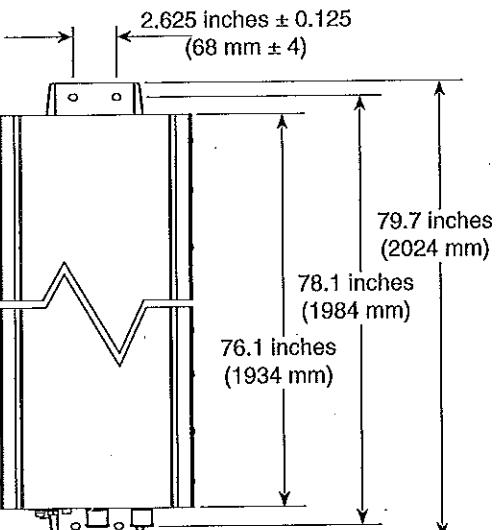
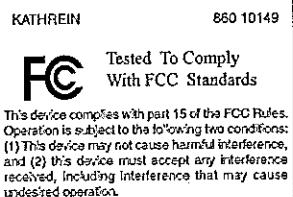
Mounting Brackets

for use with 2-point mount antennas
Mast dia. 2–4.5 inches (50–115 mm)
Weight: 4.4 lb (2 kg)

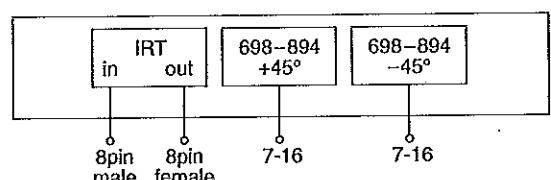
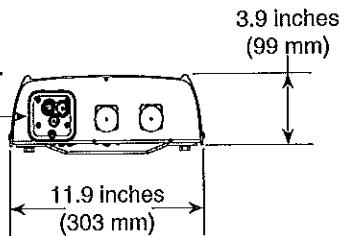


Mechanical Tilt Brackets

for use with 2-point mount antennas
Weight: 9.5 lb (4.3 kg)
(Model 850 10008)



Note: Refer to part number 860 10149 for the specifications of the remote control actuator.



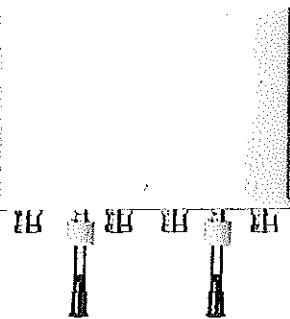
Order Information:

Model	Description
800 10735V01	Antenna with mounting bracket 0°–10° electrical downtilt
800 10735V01K	Antenna with mounting bracket and mechanical tilt bracket 0°–10° electrical downtilt

* Mechanical design is based on environmental conditions as stipulated in TIA-222-G-2 (December 2009) and/or ETS 300 019-1-4 which include the static mechanical load imposed on an antenna by wind at maximum velocity. See the Engineering Section of the catalog for further details.

Product Specifications

COMMSCOPE®



HBXX-6516DS-VM | HBXX-6516DS-A2M

Single Band Quad Port Antenna, 1710–2180 MHz, 65° horizontal beamwidth, RET compatible

- Each DualPol® array can be independently adjusted for greater flexibility
- Excellent gain, VSWR, front-to-back ratio, and PIM specifications for robust network performance
- Ideal choice for site collocations and tough zoning restrictions
- Great solution to maximize network coverage and capacity

Electrical Specifications

Frequency Band, MHz	1710–1880	1850–1990	1920–2180
Gain, dBi	17.7	18.0	18.0
Beamwidth, Horizontal, degrees	67	66	64
Beamwidth, Vertical, degrees	7.5	7.0	6.6
Beam Tilt, degrees	0–10	0–10	0–10
USLS (First Lobe), dB	18	18	18
Front-to-Back Ratio at 180°, dB	30	30	30
CPR at Boresight, dB	22	22	21
CPR at Sector, dB	8	9	9
Isolation, dB	30	30	30
VSWR Return Loss, dB	1.4 15.6	1.4 15.6	1.4 15.6
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350
Polarization	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm

Electrical Specifications, BASTA*

Frequency Band, MHz	1710–1880	1850–1990	1920–2180
Gain by all Beam Tilts, average, dBi	17.2	17.2	17.5
Gain by all Beam Tilts Tolerance, dB	±0.3	±0.3	±0.5
	0 ° 17.0	0 ° 17.1	0 ° 17.4
Gain by Beam Tilt, average, dBi	5 ° 17.3	5 ° 17.4	5 ° 17.7
	10 ° 17.0	10 ° 17.0	10 ° 17.2
Beamwidth, Horizontal Tolerance, degrees	±2.7	±2.3	±3.5
Beamwidth, Vertical Tolerance, degrees	±0.5	±0.4	±0.4
USLS, beampeak to 20° above beampeak, dB	18	19	19
Front-to-Back Total Power at 180° ± 30°, dB	26	26	26
CPR at Boresight, dB	22	22	22
CPR at Sector, dB	9	9	9

* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, download the whitepaper [Time to Raise the Bar on BSAs](#).

General Specifications

Operating Frequency Band	1710 ~ 2180 MHz
Antenna Type	Sector
Band	Single band
Performance Note	Outdoor usage

Product Specifications

COMMSCOPE®

HBXX-6516DSVTM | HBXX-6516DSA2M

Mechanical Specifications

RF Connector Quantity, total	4
RF Connector Quantity, high band	4
RF Connector Interface	7-16 DIN Female
Color	Light gray
Grounding Type	RF connector inner conductor and body grounded to reflector and mounting bracket
Radiator Material	Low loss circuit board
Radome Material	PVC, UV resistant
RF Connector Location	Bottom
Wind Loading, frontal	419.0 N @ 150 km/h 94.2 lbf @ 150 km/h
Wind Loading, lateral	113.0 N @ 150 km/h 25.4 lbf @ 150 km/h
Wind Loading, rear	488.0 N @ 150 km/h 109.7 lbf @ 150 km/h
Wind Speed, maximum	241 km/h 150 mph

Dimensions

Length	1297.0 mm 51.1 in
Width	305.0 mm 12.0 in
Depth	166.0 mm 6.5 in
Net Weight, without mounting kit	13.9 kg 30.6 lb

Remote Electrical Tilt (RET) Information

Model with Factory Installed AISG 2.0 Actuator HBXX-6516DS-A2M

Packed Dimensions

Length	1427.0 mm 56.2 in
Width	402.0 mm 15.8 in
Depth	292.0 mm 11.5 in
Shipping Weight	23.5 kg 51.8 lb

Regulatory Compliance/Certifications

Agency	Classification
RoHS 2011/65/EU	Compliant by Exemption
China RoHS SJ/T 11364-2006	Above Maximum Concentration Value (MCV)
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system



Included Products

600899A-2 — Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket

Product Specifications

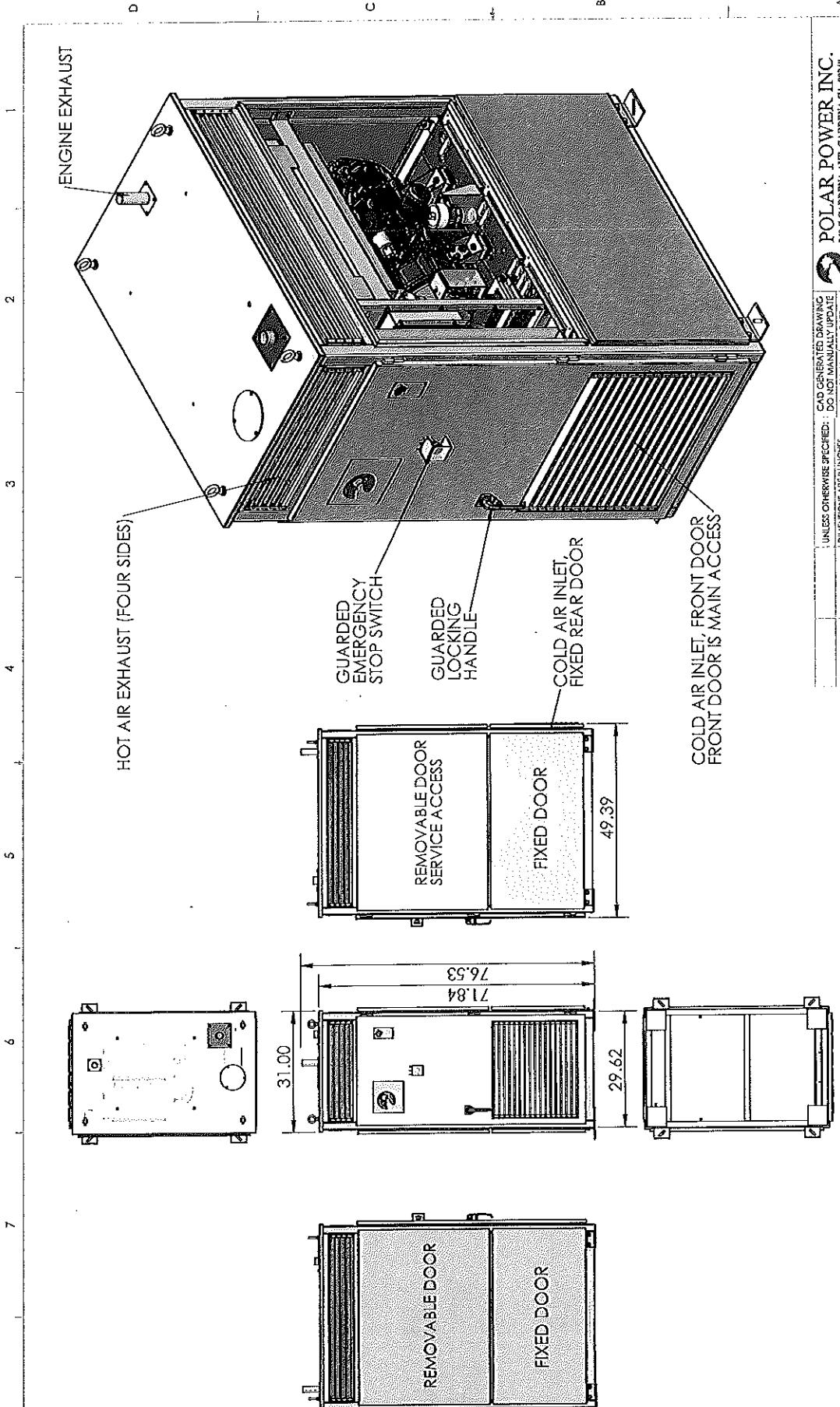
COMMSCOPE®

HBXX-6516DS-VTM | HBXX-6516DSA2M

set and one bottom bracket set.

* Footnotes

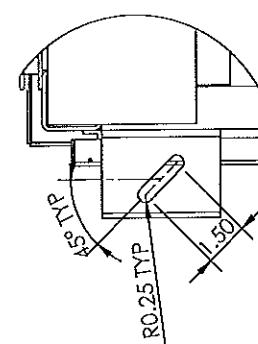
Performance Note Severe environmental conditions may degrade optimum performance



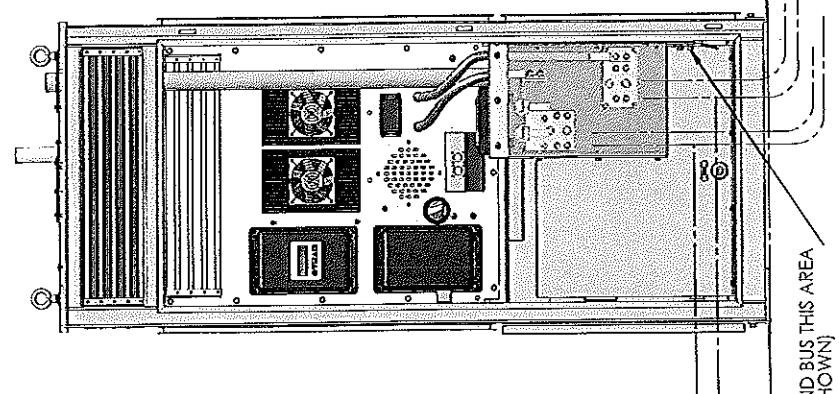
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		FRACTIONAL NOTES:	DECIMAL NOTES:	ENG APPR:	MFG APPR:	DWG. NO.: B	
INITIAL RELEASE	BY: ECO#	DATE: 7	NOT ASST:	USED ON:	APPLICATION:	SCALE: 1:24	SHEET 1 OF 4
DESCRIPTION						3	2
						4	
						5	
						6	
						7	
						8	

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INSTALLATION FOOTPRINT, BOTTOM VIEW



DETAIL A
SCALE 1 : 4



TYP ELECTRICAL PENETRATION

FRONT DOOR
REMOVED
FOR CLARITY

[T SHOWN]

UNLESS OTHERWISE SPECIFIED:		CAD GENERATED DRAWING DO NOT MANUALLY UPDATE	
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TOLERANCES ARE:	DRWNSH	AVG:	1/2"
DECIMALS: 0.0000	CNC:	1/2"	CHECKED
FRACTIONS: 1/16" 1/32" 1/64"	X-ROBOT	X-ROBOT	X-ROBOT
MATERIAL:	ENG APPR.	MATERIAL	REV
FINISH:	MFG APPR.	USED ON	DWG. NO.
-	-	NEXT ASY	B
-	-	APPLICATION	88-25-0603 A-1
-	-	DO NOT SCALE DRAWING	SCALE 1:24 WEIGHT:
-	-	Q.A.	SHEET 2 OF 4

POLAR POWER INC.
301 E GARDENIA AVE, GARDENIA, CA 90248

A
TITLE: ALUMINUM VERTICAL
ENCLOSURE, 72 IN

REV

DWG. NO.

SIZE

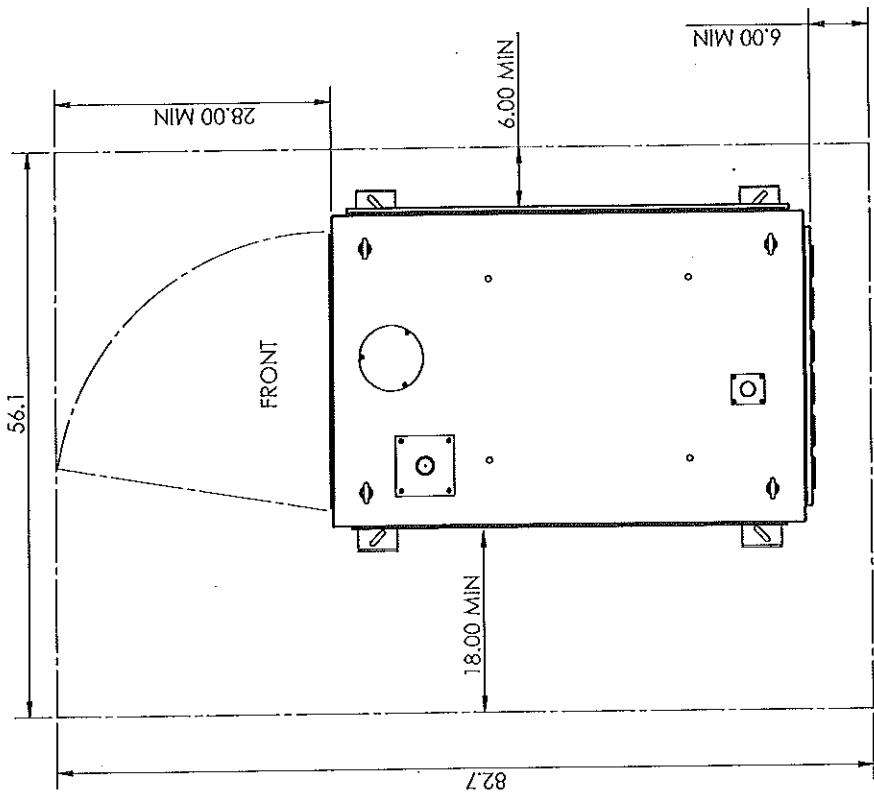
B

88-25-0603 A-1

SCALE 1:24 WEIGHT:

SHEET 2 OF 4

ALLIGATION FOOTPRINT, PLAN VIEW



D C B A

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APPROVED DATE	APPROVED DATE
249 GARDENIA AVE, GARDENA, CA 90248	DATE
2/22/2015	2/22/2015
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COMMENTS	POLAR POWER INC.
DRAWN BY	249 GARDENIA AVE, GARDENA, CA 90248
ANGLE CHECKED	ALUMINUM VERTICAL
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XCHECKED	SIZE
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ECOR	1
BY	2
DATE	3
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ALCATEL-LUCENT B13 RRH 4x30

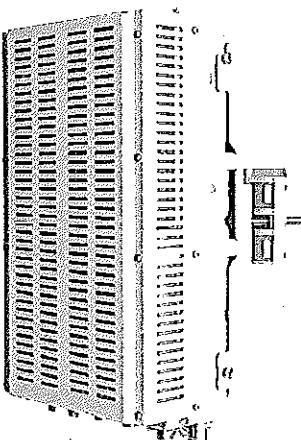
Alcatel-Lucent B13 Remote Radio Head 4x30 is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

Supporting 2Tx/4Tx MIMO and 4-way Rx diversity, Alcatel-Lucent B13 RRH 4x30 allows operators to have a compact radio solution to deploy LTE in the 700U band (700 MHz, 3GPP band 13), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B13 RRH 4x30 product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It also supports 4-way Rx diversity at 10MHz instantaneous bandwidth.

The Alcatel-Lucent B13 RRH 4x30 is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B13 RRH 4x30 easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

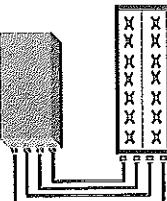


FEATURES

- Supporting LTE in 700 MHz band (700U, 3GPP band 13)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 10MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

BENEFITS

- Compact to reduce additional footprint when adding LTE in 700U band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



4x30W with 4T4R
or
2x60W with 2T4R

Can be switched between
modes via SW w/o site
visit

TECHNICAL SPECIFICATIONS

	Features & performance
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R by SW)
Frequency band	U700 (C) (3GPP bands 13): DL: 746 - 756 MHz / UL: 777 - 787 MHz
Instantaneous bandwidth - #carriers	10MHz – 1 LTE carrier (in 10MHz occupied bandwidth)
LTE carrier bandwidth	10 MHz
RF output power	2x60W or 4x30W (by SW)
Noise figure = RX Diversity/scheme	2 dB typ. (<2.5 dB max) – 2 or 4 way Rx diversity
Sizes (HxWxD) in mm (in.)	530 x 300 x 190 (20.9" x 11.8" x 7.5") (with solar shield); 525 x 290 x 175 (20.7" x 11.4" x 6.9") (w/o solar shield)
Volume in L	30.2 (with solar shield)
Weight in kg (lb) (w/o mounting HW)	25.2kg (55.6lb) (with solar shield)
DC voltage range	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	520W typical @100% RF load (in 2Tx or 4Tx mode); 380W @50% RF load; Add 50W for 2A*24V for AISG
Environmental conditions	-40°C (-40°F) / +55°C (+131°F)
Wind load (@150km/h or 93mph)	Frontal: <200N / Lateral :<150N
Antenna ports	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
AISG interfaces	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports – 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

Alcatel-Lucent Remote Radio Head B25 RRH 4x30-4R is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

Supporting 2Tx/4Tx MIMO and 4-way Rx diversity, Alcatel-Lucent B25 RRH4x30-4R allows operators to have a compact radio solution to deploy LTE in the PCS band (1.9 GHz, 3GPP band 25), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B25 RRH4x30-4R product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity, LTE carriers from 3MHz up to 20MHz and up to 65MHz instantaneous bandwidth.

The Alcatel-Lucent B25 RRH4x30-4R is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

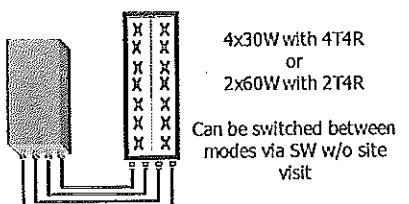
Its compactness and slim design makes the Alcatel-Lucent B25 RRH4x30-4R easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

FEATURES

- Supporting LTE in 1.9 GHz band (PCS, 3GPP band 2 & 25)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 3, 5, 10, 15 or 20MHz LTE carrier with 4Rx Diversity
- Up to 4 carriers anywhere in 65MHz instantaneous bandwidth
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

BENEFITS

- Compact to reduce additional footprint when adding LTE in PCS band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Full flexibility for multiple carriers operation over entire PCS spectrum
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



TECHNICAL SPECIFICATIONS

Features & performance	
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R by SW)
Frequency band	PCS-G (3GPP bands 2 & 25): DL: 1930 - 1995 MHz / UL: 1850 - 1915 MHz
Instantaneous bandwidth - #carriers	65MHz - Up to 4 LTE carriers (in 40MHz occupied bandwidth)
LTE carrier bandwidth	3, 5, 10, 15 or 20 MHz
RF output power	2x60W or 4x30W (by SW)
Noise figure - RX Diversity scheme	2.5 dB typ. (<3 dB max) – 2 or 4 way Rx diversity
Sizes (HxWxD) in mm (in.)	544 x 305 x 184 (21.4" x 12.0" x 7.2") (with solar shield)
Volume in L	30.5 (with solar shield)
Weight in kg (lb) (w/o mounting HW)	23 (51) (with solar shield)
DC voltage range	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	520W typical @100% RF load
Environmental conditions	-40°C (-40°F) / +55°C (+131°F) IP65
Wind load (@150km/h or 93mph)	Frontal: <200N / Lateral : <150N
Antenna ports	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
AISG interfaces	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports – 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

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ALCATEL-LUCENT B66A RRH4x45

The Alcatel-Lucent B66a Remote Radio Head 4x45 is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering. Its operational range covers beyond that of B4 (AWS) and B10 (AWS+).

Supporting 2Tx/4Tx MIMO and 2-way/4-way Rx diversity, the Alcatel-Lucent B66a RRH4x45 allows operators to have a compact radio solution to deploy LTE in the 2100 band (3GPP band 4, 10, and 66), providing them with the means to achieve high capacity, high quality, high reliability, large instantaneous bandwidth, and high coverage with minimum site requirements.

The Alcatel-Lucent B66a RRH4x45 product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x90W or 4x45W RF output power. It also supports 4-way Rx diversity at the 70 MHz instantaneous bandwidth.

The Alcatel-Lucent B66a RRH4x45 is a compact (near zero-footprint) solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

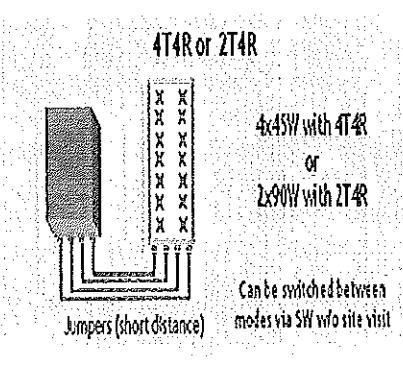
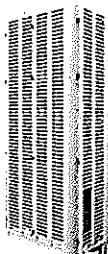
Its compactness and slim design makes the Alcatel-Lucent B66a RRH4x45 easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

FEATURES

- Supporting LTE in 2110 - 2180 MHz band/DL, 1710-1780MHz/UL (3GPP band 4, 10, and 66a)
- LTE 2Tx or 4Tx MIMO (SW selectable)
- Configuration: 2T2R/2T4R/4T4R
- Output power: Up to 2x90W or 4x45W (SW configurable)
- 70MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

BENEFITS

- Compact to reduce additional footprint when adding LTE in AWS 1-3 band
- Selection of MIMO configuration (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through 4Tx MIMO
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



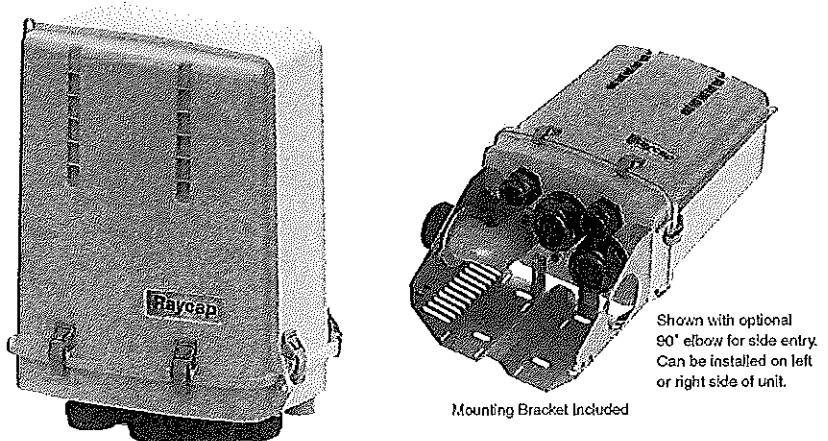
TECHNICAL SPECIFICATIONS

Features & Performance	
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R selectable by SW)
Frequency band	AWS 1-3, B4/B66a DL: 2110-2180 MHz / UL: 1710-1780 MHz
Instantaneous bandwidth - #carriers	70 MHz – 4 LTE MIMO carriers (in 70 MHz occupied bandwidth)
LTE carrier bandwidth	5, 10, 15, 20 MHz
RF output power	2x90W or 4x45W (selectable by SW)
Noise figure – RX Diversity scheme	2 dB typical (<2.5 dB max) – 2 or 4 way Rx diversity
Receiver Sensitivity (FRC A1-3)	-104.5 dBm maximum
Sizes (HxWxD) in mm (in.)	655x299x182 (25.8x11.8x7.2) (with solar shield) 640x290x160 (25.2x11.4x6.3) (without solar shield)
Volume in Liters	35.5 (with solar shield) 29.7 (without solar shield)
Weight in kg (lb) (w/o mounting HW)	25.8kg (56.8lb) (with solar shield)
DC voltage range	Nominal: -48V, -40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	750W typical @100% RF load (in 2Tx or 4Tx mode); Add 58W for 2A*29V for AISG
Environmental conditions	-40°C (-40°F) / +55°C (+131°F) UL50E Type 4 Enclosure
Wind load (@150km/h or 93mph)	250N (56lb) Frontal/150N (34lb) Lateral
Antenna ports	4 ports 4.3-10 female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate 7, 9.8 Gbps) SFP: SMDF (HW supports also SMSF and MMDF)
AISG interfaces	1 AISG 2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-487 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27 / FCC Part 15 / GR-3178-CORE

DATA SHEET

DC Surge Protection for RRH/Integrated Antenna Radio Head
RxxDC-4750-PF-48 • RxxDC-3103-PF-48 •
RxxDC-3315-PF-48
Tower / Base / Rooftop / Rooftop Distribution Models

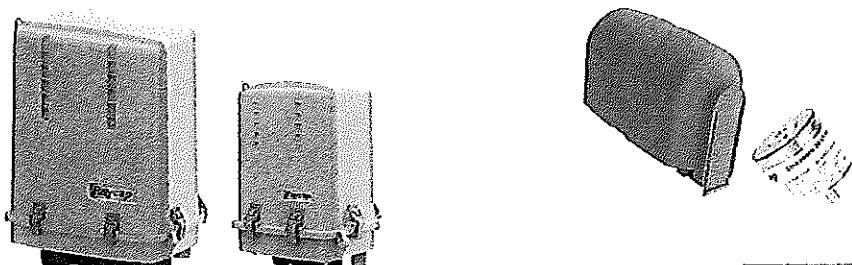
Raycap's flexible Tower, Base Stations and Rooftop protection and Distribution products provide protection for up to 6 Remote Radio Heads/Integrated Antennas. The solutions mitigate the risk of damage due to lightning and provide high levels of availability and reliability to radio equipment.

**Features**

- Employs the Strikesorb® 30-V1-HV Surge Protective Device (SPD) specifically designed for the Remote Radio Head (RRH) installation environment and certified for use in DC applications and at low DC operating voltages (48V).
- The Strikesorb 30-V1-HV is a Class I SPD, certified by VDE per the IEC 61643-1 standard as suitable for installation in areas where direct lightning exposure is expected. Strikesorb 30-V1-HV is able to withstand direct lightning currents of up to 5kA (10/350) and induced surge currents of up to 60kA (8/20).
- Provides very low let through / clamping voltage - unique for a Class I product - as it does not employ spark gaps or other switching elements. Strikesorb offers unique protection levels to the RRH equipment as well as the Base Band Units.
- Alarms for SPD sacrifice, Moisture detection and Intrusion.
- Fully recognized to the UL 1449 3rd Edition Safety Standard.
- Patent pending design

Benefits

- Offers unique maintenance-free protection against direct lightning currents.
- Protects up to 6 Remote Radio Heads and connects up to 12 fiber pairs.
- Utilizes an IP 67 rated enclosure, allowing for indoor or outdoor installation on a roof or tower top.
- Configurable cable ports are designed to accommodate varying diameters of hybrid (combined power and fiber optic) or standard cables with diameters up to 2" (will fit most standard 1 1/8" coax class cables) depending upon port configuration.
- Lightweight aerodynamic design provides maximum flexibility for tower top installation.
- Companion to the RxxDC-4291-PF-48 / RxxDC-1064-PF-48 (Sector) models.



Tower / Base / Rooftop /
Rooftop Distribution Models
RxxDC-4750-PF-48
RxxDC-3103-PF-48
RxxDC-3315-PF-48

Companion Sector Models:
RxxDC-4291-PF-48
RxxDC-1064-PF-48

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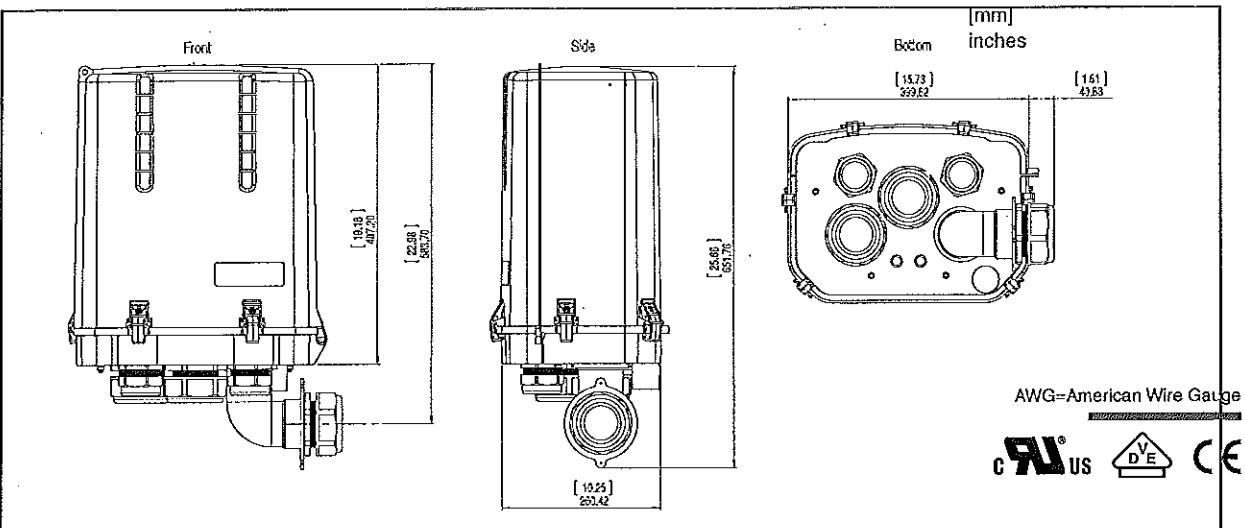
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SPECIFICATIONS

DC Surge Protection for RRH/Integrated Antenna Radio Head
RxxDC-4750-PF-48 • RxxDC-3103-PF-48 •
RxxDC-3315-PF-48
Tower / Base / Rooftop / Rooftop Distribution Models

Electrical

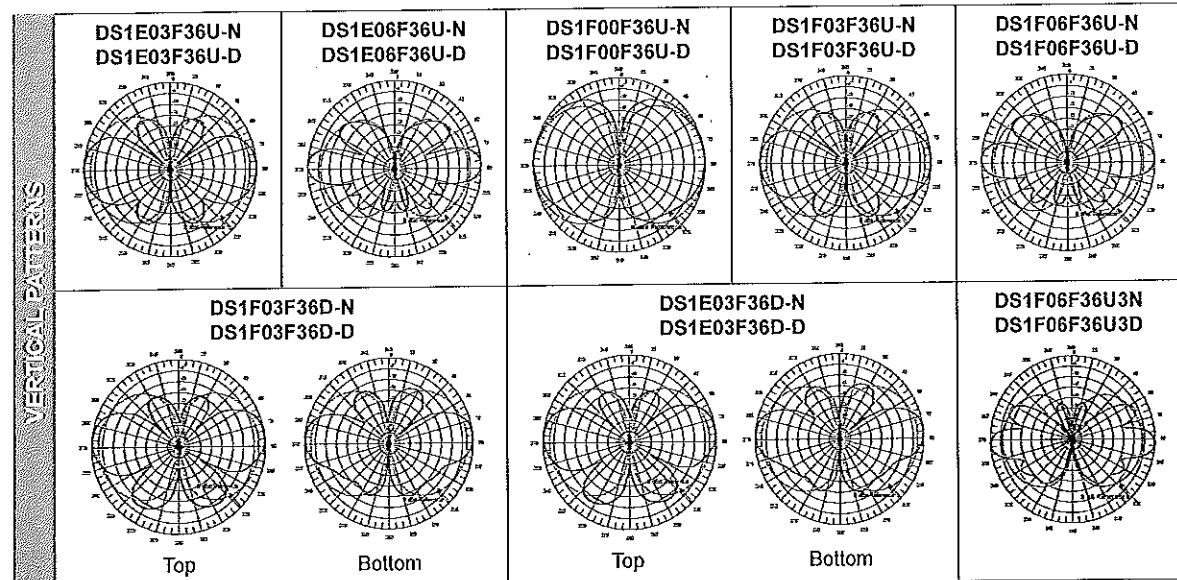
Model Numbers	RxxDC-4750-PF-48	RxxDC-3103-PF-48	RxxDC-3315-PF-48
Nominal Operating Voltage	48 VDC	48 VDC	48 VDC
Nominal Discharge Current [I_{pk}]	20 kA 8/20 μ s	20 kA 8/20 μ s	20 kA 8/20 μ s
Maximum Surge Current [I_{pk}]	60 kA 8/20 μ s	60 kA 8/20 μ s	60 kA 8/20 μ s
Maximum Impulse (Lightning) Current per IEC 61643-1	5 kA 10/350 μ s	5 kA 10/350 μ s	5 kA 10/350 μ s
Maximum Continuous Operating Voltage [U]	75 VDC	75 VDC	75 VDC
Voltage Protection Rating (VPR) per UL 1449 3rd Edition	400V	400V	400V
Protection Class as per IEC 61643-1	Class I	Class I	Class I
SPD Alarm	upon sacrifice	upon sacrifice	upon sacrifice
Intrusion Sensor	microswitch	microswitch	microswitch
Moisture Sensor	infrared moisture detector	infrared moisture detector	infrared moisture detector
Strikesorb Module Type	No Strikesorb modules installed	30-V1-HV Strikesorb modules installed to protect 3 Remote Radio Heads	30-V1-HV Strikesorb modules installed to protect 6 Remote Radio Heads
Suppression Connection Method	Compression lug, #14 - #2/0 AWG (2.5 mm ² - 70 mm ²) Copper; #12 - #2/0 AWG (4 mm ² - 70 mm ²) Aluminum		
Fiber Connection Method	LC-LC Single mode		
Pressure Equalizing Vent	Gore TM Vent		
Environmental Rating	IP 67		
Operating Temperature	-40° C to +80° C		
UV Resistant	Yes		
Weight	System: 16.0 lbs (7.25 kg) Mount: 5.5 lbs (2.49 kg) Total: 21.5 lbs (9.75 kg)	System: 18.7 lbs (8.48 kg) Mount: 5.5 lbs (2.49 kg) Total: 24.2 lbs (10.98 kg)	System: 21.4 lbs (9.70 kg) Mount: 5.5 lbs (2.49 kg) Total: 26.9 lbs (12.20 kg)
Combined Wind Loading	150mph (sustained): 200 lbs (889.6 N)		
Strikesorb modules are compliant to the following Surge Protective Device (SPD) Standards			
Standards	ANSI/UL 1449 3rd Edition		
	IEEE C62.41		
	NEMA LS-1, IEC 61643-1:2005 2nd Edition (Class I Protection)		
	IEC 61643-12		
	EN 61643-11:2002 (including A11:2007)		

Product Diagram

dbSpectra

VHF Omni Antennas (140-164 MHz)

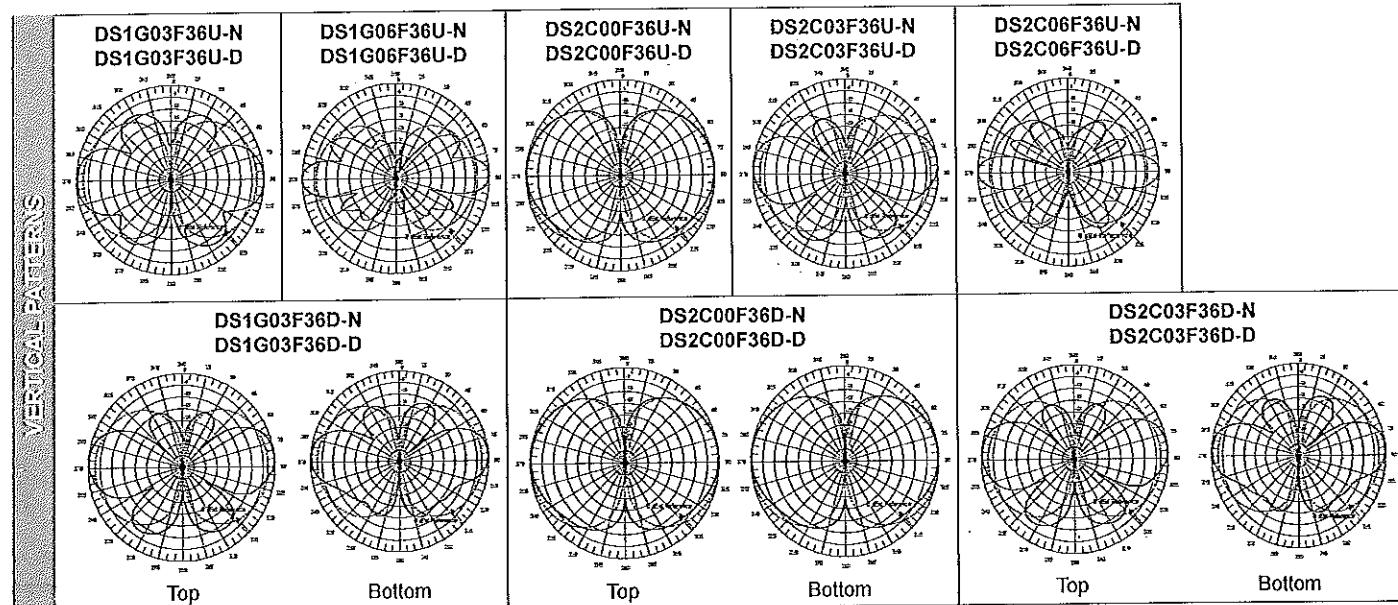
Model Number	140-150 MHz						150-164 MHz									
	DS1E03F36U-N	DS1E03F36U-D	DS1E06F36U-N	DS1E06F36U-D	DS1E03F36D-N	DS1E03F36D-D	DS1F00F36U-N	DS1F00F36U-D	DS1F03F36U-N	DS1F03F36U-D	DS1F06F36U-N	DS1F06F36U-D	DS1F03F36D-N	DS1F03F36D-D	DS1F06F36U3N	DS1F06F36U3D
Input Connector	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN	N(F)	7/16 DIN
Type	Single		Single		Dual			Single			Dual		Beamtilt			
Bandwidth, MHz	10		10		10		14	14	14	14	14		14			
Power, Watts	500		500		350		500	500	500	350	500		500			
Gain, dBd	3		6		3		0	3	6	3	6		6			
Horizontal Beamwidth, degrees	360		360		360		360	360	360	360	360		360			
Vertical Beamwidth, degrees	30		16		30		65	30	16	30	16		16			
Beam Tilt, degrees	0		0		0		0	0	0	0	3 Down					
Isolation (minimum), dB	N/A		N/A		30		N/A	N/A	N/A	30	N/A					
Number of Connectors	1		1		2		1	1	1	2	1					
Flat Plate Area, ft ² (m ²)	2.5 (0.23)		4.58 (0.43)		4.1 (0.38)		1.63 (0.15)	2.53 (0.24)	4.38 (0.41)	4.5 (0.42)	4.38 (0.41)					
Lateral Windload Thrust, lbf(N)	105 (467)		172 (765)		169 (752)		60 (267)	95 (423)	164 (730)	169 (752)	164 (730)					
Survival Wind Speed without ice, mph(kph) with 0.5" radial ice, mph(kph)	120 (193) 100 (161)		72 (116) 52 (84)		75 (121) 65 (105)		200 (322) 161 (260)	110 (177) 93 (150)	75 (121) 60 (97)	75 (121) 65 (105)	75 (121) 60 (97)					
Mounting Hardware Included	DSH3V3N		DSH3V3N		DSH3V3N		DSH2V3R	DSH3V3R	DSH3V3N	DSH3V3N	DSH3V3N					
Length, ft(m)	14.9 (4.5)		22.9 (7)		24.3 (7.4)		8.2 (2.5)	12.6 (3.8)	21.9 (6.7)	22.3 (6.8)	21.9 (6.7)					
Radome O.D., in(cm)	3 (7.6)		3 (7.6)		3 (7.6)		3 (7.6)	3 (7.6)	3 (7.6)	3 (7.6)	3 (7.6)					
Mast O.D., in(cm)	2.5 (6.4)		2.5 (6.4)		2.5 (6.4)		2.5 (6.4)	2.5 (6.4)	2.5 (6.4)	2.5 (6.4)	2.5 (6.4)					
Net Weight w/o bracket, lb(kg)	43 (19.5)		65 (29.5)		70 (31.8)		20 (9.1)	37 (16.8)	60 (27.2)	63 (28.6)	60 (27.2)					
Shipping Weight, lb(kg)	73 (33.1)		95 (43.1)		100 (45.4)		40 (18.1)	67 (30.4)	90 (40.8)	93 (42.2)	90 (40.8)					



dbSpectra

VHF Omni Antennas (160-222 MHz)

Model Number	160-174 MHz						217-222 MHz											
	DS1G03F36U-N	DS1G03F36U-D	DS1G06F36U-N	DS1G06F36U-D	DS1G03F36D-N	DS1G03F36D-D	DS2C00F36U-N	DS2C00F36U-D	DS2C03F36U-N	DS2C03F36U-D	DS2C06F36U-N	DS2C06F36U-D	DS2C00F36D-N	DS2C00F36D-D	DS2C03F36D-N	DS2C03F36D-D		
Input Connector	N(F) 7/16 DIN																	
Type	Single	Single	Single	Dual			Single	Single	Single	Dual	Dual	Dual						
Bandwidth, MHz	14	14	14	14			5	5	5	5	5	5						
Power, Watts	500	500	500	350			500	500	500	350	350	350						
Gain, dBd	3	6	6	3			0	3	6	0	0	3						
Horizontal Beamwidth, degrees	360	360	360	360			360	360	360	360	360	360						
Vertical Beamwidth, degrees	30	16	16	30			60	30	16	60	60	30						
Beam Tilt, degrees	0	0	0	0			0	0	0	0	0	0						
Isolation (minimum), dB	N/A	N/A	N/A	30			N/A	N/A	N/A	30	30	30						
Number of Connectors	1	1	1	2			1	1	1	2	2	2						
Flat Plate Area, ft ² (m ²)	2.53 (0.24)	4.38 (0.41)	4.5 (0.42)				1.9 (0.18)	1.9 (0.18)	2.58 (0.24)	2.4 (0.22)	4.1 (0.38)							
Lateral Windload Thrust, lbf(N)	95 (423)	164 (730)	169 (752)				53 (236)	69 (307)	108 (480)	90 (400)	169 (752)							
Survival Wind Speed without ice, mph(kph)	110 (177)	75 (121)	75 (121)				222 (357)	172 (277)	110 (177)	130 (209)	75 (121)							
with 0.5" radial ice, mph(kph)	93 (150)	60 (97)	65 (105)				193 (311)	150 (241)	96 (154)	115 (185)	65 (105)							
Mounting Hardware included	DSH3V3R	DSH3V3N	DSH3V3N				DSH2V3R	DSH2V3R	DSH3V3N	DSH3V3R	DSH3V3N							
Length, ft(m)	12.7 (3.9)	21.9 (6.7)	22.3 (6.8)				7.7 (2.3)	9.9 (3)	18.1 (5.5)	13.6 (4.1)	24.3 (7.4)							
Radome O.D., in(cm)	3 (7.6)	3 (7.6)	3 (7.6)				3 (7.6)	3 (7.6)	3 (7.6)	3 (7.6)	3 (7.6)							
Mast O.D., in(cm)	2.5 (6.4)	2.5 (6.4)	2.5 (6.4)				2.5 (6.4)	2.5 (6.4)	2.5 (6.4)	2.5 (6.4)	2.5 (6.4)							
Net Weight w/o bracket, lb(kg)	37 (16.8)	60 (27.2)	63 (28.6)				19 (8.6)	26 (11.8)	47 (21.3)	40 (18.1)	70 (31.8)							
Shipping Weight, lb(kg)	67 (30.4)	90 (40.8)	93 (42.2)				39 (17.7)	56 (25.4)	77 (34.9)	70 (31.8)	100 (45.4)							

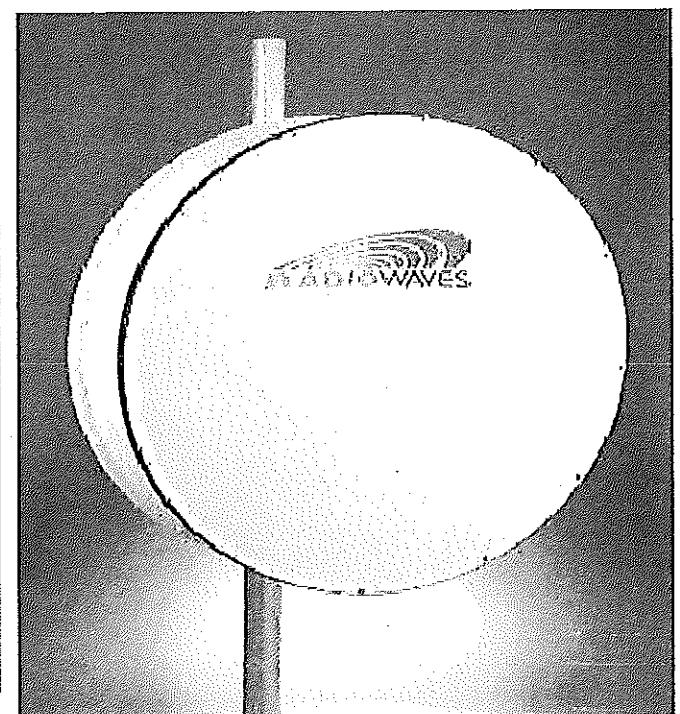




High Performance Series for 4.4-5.0 GHz Frequencies

Key Features

- High Performance antennas minimize interference as they have more stringent radiation side lobe and front-to-back suppression characteristic
- Lightweight and rugged design
- Easily installed with our superior mounting system included with the antenna
- RF connector: "N" female connector. Some models are available with 7/16 DIN Connector. Please call the factory for availability
- Our industry leading 5-year warranty
- Radome is included
- Single (HP) and Dual (HPD) polarization are available



Antenna Specifications, Electrical (typical)

Model Number	Diameter ft. (m)	Frequency GHz	Gain (dBi)			3dB BW degs	X-Pol Rejection, dB	F/B Ratio dB	VSWR, Max (R.L., dB)	Antenna Weight
HP2-4.7	2 (0.6)	4.4-5.0	25.8	26.4	29.6	7.1 deg.	28 dB	48 dB	1.5:1 (14.0)	27 lbs. (12.3 kg)
HP3-4.7	3 (0.9)	4.4-5.0	29.2	29.8	30.3	4.7 deg.	30 dB	52 dB	1.5:1 (14.0)	50 lbs. (22.7 kg)
HP4-4.7	4 (1.2)	4.4-5.0	31.8	32.4	32.9	3.6 deg.	30 dB	54 dB	1.5:1 (14.0)	85 lbs. (38.3 kg)
HP6-4.7	6 (1.8)	4.4-5.0	34.8	35.4	35.3	2.6 deg.	30 dB	57 dB	1.5:1 (14.0)	251 lbs. (113.0 kg)
HP8-4.7	8 (2.4)	4.4-5.0	48.2	38.8	39.3	1.8 deg.	30 dB	61 dB	1.5:1 (14.0)	424 lbs. (194.5 kg)
HPD2-4.7	2 (0.6)	4.4-5.0	25.8	26.4	26.9	7.1 deg.	28 dB	48 dB	1.5:1 (14.0)	27 lbs. (12.3 kg)
HPD3-4.7	3 (0.9)	4.4-5.0	29.2	29.8	30.3	4.7 deg.	30 dB	52 dB	1.5:1 (14.0)	50 lbs. (22.7 kg)
HPD4-4.7	4 (1.2)	4.4-5.0	31.8	32.4	32.9	3.6 deg.	30 dB	54 dB	1.5:1 (14.0)	85 lbs. (38.3 kg)
HPD6-4.7	6 (1.8)	4.4-5.0	34.8	35.4	35.9	2.6 deg.	30 dB	57 dB	1.5:1 (14.0)	251 lbs. (113.0 kg)
HPD8-4.7	8 (2.4)	4.4-5.0	38.2	38.8	39.3	1.8 deg.	30 dB	61 dB	1.5:1 (14.0)	424 lbs. (194.5 kg)

Note: LMR jumpers and Side Struts available from Radio Waves